

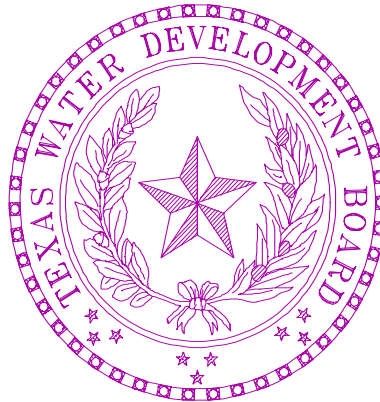
VOLUMETRIC SURVEY OF EAGLE MOUNTAIN LAKE

Prepared for:

Tarrant Regional Water District

In cooperation with the

United States Army Corps of Engineers



**Prepared by
Texas Water Development Board**

August 21, 2001

Texas Water Development Board

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Texas Water Development Board

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TABLE OF CONTENTS

INTRODUCTION.....	1
LAKE HISTORY AND GENERAL INFORMATION.....	2
SURVEYING TECHNOLOGY	4
PRE-SURVEY PROCEDURES.....	5
SURVEY PROCEDURES	6
Equipment Calibration and Operation	6
Field Survey	7
Data Processing.....	8
RESULTS	10
SUMMARY AND COMPARISONS.....	10
REFERENCES	11

APPENDICES

APPENDIX A - VOLUME TABLE
APPENDIX B - AREA TABLE
APPENDIX C - ELEVATION-VOLUME GRAPH
APPENDIX D - ELEVATION-AREA GRAPH
APPENDIX E - CHANGE IN VOLUME GRAPH
APPENDIX F - CHANGE IN AREA GRAPH
APPENDIX G - CROSS-SECTION GRAPHS
APPENDIX H - DEPTH SOUNDER ACCURACY

LIST OF FIGURES

FIGURE 1 - LOCATION MAP
FIGURE 2 - LOCATION OF SURVEY DATA
FIGURE 3 - SHADED ELEVATION RANGES
FIGURE 4 - SHADED DEPTH RANGES
FIGURE 5 - CONTOUR MAP

EAGLE MOUNTAIN LAKE VOLUMETRIC SURVEY REPORT

INTRODUCTION

Staff of the Surface Water Section of the Texas Water Development Board (TWDB) conducted a volumetric survey of Eagle Mountain Lake between April 18 and April 26, 2000. The primary purpose of this survey was to determine the volume of the lake at the pool elevation during the time of the survey. The lake elevation varied between 643.19 feet and 643.43 feet during the survey, and was approximately 5.8 feet below the conservation pool elevation of 649.1 feet. Subsequent data was collected on May 16 and May 17, 2001 when the lake elevation was 649.05 feet and 649.06 feet respectively. This report supercedes the January 26, 2001 report of the same name in which comparison of results was made only to elevation 643.0 feet. Comparisons of results between the present survey and past surveys are made for elevation 649.1 feet later in this report. Results from this survey will serve as a basis for comparison to future surveys to allow the location and rates of sediment deposition to be determined. Survey results are presented in the following pages in both graphical and tabular form.

The vertical datum used during this survey is that used by the United States Geological Survey (USGS) for the reservoir elevation gage at Eagle Mountain Lake (08045000 EAGLE MOUNTAIN RESERVOIR ABOVE FORT WORTH, TX). The datum for this gage is reported as mean sea level (msl) (USGS, 1999). Thus, elevations are reported here, according to the same datum, in feet above mean sea level (msl). Volume and area calculations in this report are referenced to water levels provided by the USGS gage.

Prior surveys of Eagle Mountain Lake were conducted in 1968, by the U.S. Army Corps of Engineers, and in 1988, by Freese and Nichols. The volume at conservation storage elevation (649.1 feet) was found to be 190,460 acre-feet in 1968 (TWDB, 1973), and 178,440 feet in

1988 (Freese and Nichols, 1988). The corresponding surface areas at that elevation were found to be 9,200 acres in 1968, and 9,030 acres in 1988.

LAKE HISTORY AND GENERAL INFORMATION

Historical information on Eagle Mountain Lake was obtained from TWDB (1966), TWDB (1973), and a sedimentation survey report prepared for the Tarrant County Water Control and Improvement District No. 1 by Freese and Nichols (1988). The Tarrant Regional Water District (TRWD) owns the water rights to Eagle Mountain Lake. TRWD also owns, operates and maintains associated Eagle Mountain Dam. The lake is located on the West Fork Trinity River (Trinity River Basin) in Tarrant County, 14 miles northwest of Fort Worth, Texas (Figure 1). Records indicate the drainage area is approximately 1,970 square miles. At conservation pool elevation (649.1) feet the lake has approximately 83 miles of shoreline and is 11 miles long. The widest point of the lake is approximately 3.25 miles and is located about 0.5 miles upstream of the dam.

One of the main functions of Eagle Mountain Lake is to regulate floodwaters on the West Fork Trinity River in coordination with the operations of Lake Bridgeport, which is located upstream of Eagle Mountain Lake. The Board of Water Engineers issued Water Rights Permit No. 1074 (Application No. 1145) to the Tarrant County Water Control and Improvement District No. 1 on May 1, 1928. The permit authorized the District to construct a dam across the West Fork Trinity River in Tarrant County and to impound 210,000 acre-feet of water. Annual diversions of 162,000 acre-feet of water for municipal, industrial and irrigation were granted. Permit No. 1254 was granted to the District on February 28, 1938 as an amendment to Permit No. 1074. The permit basically included “recreational and pleasure” use of the water in Eagle Mountain Lake. Permit No. 1074 was again amended on September 30, 1953 in Permit No. 1682. There were no changes in the amount of impoundment or diversion, but the amendment described in more detail the municipal and irrigation uses of the diverted water within the boundaries of the District. The Texas Water Commission issued a special minute order dated October 9, 1962 that approved the application for amendment to the original construction plans in Permit No. 1074. The plans included the construction of an emergency spillway. In order to improve the discharge characteristics and increase the discharge capacity, the construction would consist of excavation and fill to widen and realign the original

country spillway. The modifications to the emergency spillway added a six-foot-high fuse plug that would increase the floodwater retarding capabilities of the facility without jeopardizing the integrity of the dam. This design would increase the surcharge storage by 106,640 acre-feet. Permit No. 1074A, dated September 20, 1965, reduced the original 93,000 acre-feet of water releases from Lake Bridgeport to 90,000 acre-feet of water. The 162,000 acre-feet of water originally authorized to be impounded in Eagle Mountain Lake were also reduced to 159,600 acre-feet of water. Authorization was granted in the amended Permit No. 1073B (Lake Bridgeport) to reduce the releases from Lake Bridgeport to Eagle Mountain Lake to 86,000 acre-feet of water annually. The Texas Water Commission on April 5, 1985 issued Certificate of Adjudication No. 08-3809 to the Tarrant County Water Control and Improvement District No. 1. The certificate granted the District the right to impound not to exceed 210,000 acre-feet of water in a reservoir known as Eagle Mountain Lake. The owner was authorized to divert, not to exceed 159,600 acre-feet of water per annum for subsequent downstream diversion from the West Fork Trinity River for municipal and industrial purposes. Diversion could also be used for irrigation of land within the boundaries of the District. On May 15, 1985 an amendment was issued for Certificate of Adjudication No. 08-3809(A). Permission was granted to use 1,105 acre-feet of water, previously designated for municipal use, for mining purposes.

Construction for Eagle Mountain Lake and Dam started on January 23, 1930. The dam was completed October 24, 1932 and impoundment began on February 28, 1934. The design engineer for the project was Hawley, Freese and Nichols. The general contractor was McKenzie and Uvalde Construction Companies. The estimated cost of the dam was \$3,637,000. A second spillway was added in 1971. The project engineer was Freese, Nichols and Endress and the general contractor was Guy H. James Construction Co. The new spillway was completed on July 31, 1971 at an estimated cost of \$2,565,679.

Engineering designs (TWDB, 1974) show that the original Eagle Mountain Dam consisted of two earthfill embankments. Eagle Mountain, both the original and new concrete spillways, and the Burgess Gap emergency spillway lie between the main dam and spillway. The main dam is approximately 4,800 feet in length with a maximum height of 85 feet above the natural streambed. The dam has a 25-foot-wide crest at elevation of 682.0 feet. The spillway dam is located to the west

of the main dam and Eagle Mountain. Located within the levee are the original and new spillways. The original spillway is a concrete structure consisting of an ogee crest that is divided into four 25-foot wide bays. Three of the four bays are equipped with vertical lift gates while the fourth bay is left uncontrolled. The crest elevation is 649.1 feet. The new spillway (1971) is located 300 feet to the east of the original spillway. The new side-channel structure consists of a concrete ogee crest and forebay that discharges through a 25-foot-square conduit. The ogee crest has an elevation of 637.0 and is controlled by six roller gates, each 11.25 feet by 22.0 feet. The emergency spillway is located at Burgess Gap between the main dam and spillway dam. The emergency spillway was modified in 1965. The natural cut channel is 1,300 feet in length with a crest elevation of 670.0 feet. A fuse plug was added as part of the modifications and is six feet tall with a crest elevation of 676.0 feet.

The outlet works consist of four 48-inch outlet gates and are operated from a control house located on the upstream face (east end) of the main dam. The invert elevation for all four gates is 599.9 feet. Flows are discharged downstream of the main dam through a conduit that extends through the embankment.

SURVEYING TECHNOLOGY

The equipment used to perform the volumetric survey consists of a 23-foot aluminum tri-hull SeaArk craft with cabin, equipped with twin 90-Horsepower Honda outboard motors. (Reference to brand names throughout this report does not imply endorsement by TWDB). Installed within the enclosed cabin are a Coastal Oceanographics' Helmsman Display (for navigation), an Innerspace Technology Model 449 Depth Sounder and Model 443 Velocity Profiler, Trimble Navigation, Inc. 4000SE GPS receiver, an OmniSTAR receiver, and an on-board 486 computer. A water-cooled generator provides electrical power through an in-line uninterruptible power supply. In shallow areas and where navigational hazards (stumps) were present, a 20-foot aluminum shallow-draft flat bottom SeaArk craft with cabin and equipped with one 115-horsepower Evinrude outboard motor was used. The portable data collection equipment on-board the boat included a Knudsen 320 B/P Echosounder (depth sounder), a Trimble Navigation, Inc. 4000SE GPS receiver, an OmniSTAR receiver, and a laptop computer.

The GPS equipment, survey vessel, and depth sounder in combination provide an efficient hydrographic survey system. As the boat travels across the lake surface, the depth sounder takes approximately ten readings of the lake bottom each second. The depth readings are stored on the survey vessel's on-board computer along with the corrected positional data generated by the boat's GPS receiver. The daily data files collected are downloaded from the computer and brought to the office for editing after the survey is completed. During editing, poor-quality data is removed or corrected, multiple data points are averaged to get one data point per second, and average depths are converted to elevation readings based on the lake elevation recorded on the day the survey was performed. Accurate estimates of the lake volume can be quickly determined by building a 3-D model of the reservoir from the collected data.

PRE-SURVEY PROCEDURES

The reservoir's boundary was digitized using Environmental Systems Research Institute's (ESRI) Arcview from digital orthophoto quadrangle images (DOQ's). The DOQ's were produced by VARGIS of Texas LLC for the TEXAS Orthoimagery Program (TOP). The DOQ products produced for the Department of Information Resources and the GIS Planning Council under the Texas Orthoimagery Program reside in the public domain. More information can be obtained on the Internet at <http://www.tnris.state.tx.us/DigitalData/doqs.htm>. The map boundary was created from the Avondale and Azle, Texas DOQs. The lake elevations at the time the DOQs were photographed were 649.11 feet (January 31, 1995) and 649.03 feet (January 19, 1995). In addition, a portion of the lake's upstream boundary was digitized at elevation 649 feet with AutoCad software from USGS 7.5-minute quadrangle map for BOYD, TEX. (1960) (photo-revised 1968, photo-inspected 1973).

The DOQ graphic boundary file was transformed from UTM Zone 14 datum to NAD '83, using Environmental Systems Research Institute's (ESRI) Arc/Info PROJECT command with the NADCOM (standard conversion method within the United States) parameters. Similarly, the portion of the boundary digitized from the USGS 7.5-minute quadrangle map was transformed from NAD '27 to NAD '83.

The survey layout was designed by placing survey track lines at 500-foot intervals within the

digitized lake boundary using HYPACK software. The survey design required the use of approximately 257 survey lines along the length of the lake.

SURVEY PROCEDURES

Equipment Calibration and Operation

At the beginning of each day of the survey, the depth sounder was calibrated with the Innerspace Velocity Profiler, an instrument used to measure the variation in the speed of sound at different depths in the water column. The average speed of sound through the entire water column below the boat was determined by averaging local speed-of-sound measurements collected through the water column. The velocity profiler probe was first placed in the water to moisten and acclimate the probe. The probe was next raised to the water surface where the depth was zeroed. The probe was then gradually lowered on a cable to a depth just above the lake bottom, and then raised to the surface. During this lowering and raising procedure, local speed-of-sound measurements were collected, from which the average speed was computed by the velocity profiler. This average speed of sound was entered into the ITI449 depth sounder, which then provided the depth of the lake bottom. The depth was then checked manually with a measuring tape to ensure that the depth sounder was properly calibrated and operating correctly.

On the shallow draft boat the depth sounder was calibrated using the bar check feature in the Knudsen software program. This was accomplished by positioning the transducer over a known (measured) depth. The speed of sound was then adjusted (either higher or lower) until the displayed depths matched the known depth. The depth was then checked manually with a stadia (survey) rod to ensure that the depth sounder was properly calibrated and operating correctly.

While surveying Eagle Mountain Lake, the speed of sound in the water column ranged from 4833 feet per second to 4850 feet per second. Based on the measured speed of sound for various depths and the average speed of sound calculated for the entire water column, the depth sounder is accurate to within ± 0.2 feet. An additional estimated error of ± 0.3 feet arises from variation in boat inclination. These two factors combine to give an overall accuracy of ± 0.5 feet for any instantaneous

reading. These errors tend to be minimized over the entire survey, since some readings are positive and some are negative. Further information on these calculations is presented in Appendix E.

During the survey, the horizontal mask setting on the on-board GPS receiver was set to 10°, and the PDOP (Position Dilution of Precision) limit was set to 7 to maximize the accuracy of the horizontal positioning. An internal alarm sounds if PDOP rises above seven to advise the field crew that the horizontal position has degraded to an unacceptable level. The lake's initialization file used by the Hypack data collection program was set up to convert the collected DGPS positions to state-plane coordinates on the fly.

Field Survey

Originally, TWDB staff collected data at Eagle Mountain Lake for eight days between April 18 and April 26, 2000. According to the USGS gauging station, the lake level elevations ranged between 643.19 and 643.43 feet during that period, or approximately 5.8 feet below conservation pool elevation of 649.1. TWDB staff then returned to Eagle Mountain Lake for two days on May 16 and 17, 2001 when the lake elevations were 649.05 and 649.06 respectively. Weather during most of the data collection period consisted of sunny days, warm temperatures, and 10 to 20 mile-per-hour winds.

The survey crew was able to collect data on 200 of the 257 pre-plotted survey transects in the lake during the April 2000 survey dates. An additional 22 pre-plotted were collected during the May 2001 survey dates. Random data was collected along the shoreline and in those areas that were too restricted to drive the pre-plotted lines. Approximately 104,827 data points were collected over the 199 miles traveled. These points, shown in Figure 2, were stored digitally on the boat's computer in 290 data files.

The West Fork Trinity River meanders in a north to south direction and empties into the main body of Eagle Mountain Lake at the north end of the basin. The Dossier, Indian and Derrett Creeks empty into the lake from the east. Tributaries that join Eagle Mountain Lake from the west are the

Ash, Walnut, Swift and Briar Creeks. The east shoreline has more relief, with steep slopes cliffs and valleys, than the west bank, which has generally flat to rolling hills.

More residential and commercial development (marinas) was observed in the lower part of the main basin than in the upper end, where acreage properties and undeveloped land was found.

An analog strip-chart recording collected during the survey showed the bathymetry of the lake bottom to be fairly regular (no major drops or rises) in the main basin of the lake. Data was collected around the perimeter of Pelican Island to be used later to adjust the lake boundary. TXU Electric has an electric power generating plant located on the east bank of the lake near Pelican Island. The survey crew collected extra data around the intake structure and discharge channel for the power plant and around other intake structures. The bathymetry of the lake bottom was similar to the topography surrounding the lake. A defined channel (thalweg) for the West Fork Trinity River was evident on the analog chart as the crew traveled in a perpendicular (east and west) direction in the main basin of the lake.

The field crew encountered navigational hazards, in the form of submerged stumps and shallow depths outside the old river channel, in the upper reaches of Eagle Mountain Lake in the West Fork Trinity River. Data was collected in this area at a much slower rate using the shallow draft boat. Data collection was halted in the upper reaches of the lake that were less than one foot deep.

The collected data were stored in individual data files for each pre-plotted range line or random data collection event. These files were downloaded to diskettes at the end of each day for subsequent processing.

Data Processing

The collected data were downloaded from diskettes onto TWDB's computer network. Tape backups were made for future reference as needed. To process the data, the EDIT routine in the HYPACK Program was run on each raw data file. Data points such as depth spikes or data with missing depth or positional information were deleted from the file. A correction to account for the

lake's elevation when the data were collected was also applied to each file during the EDIT routine. During the April 2000 survey dates, the water surface fluctuated between elevation 643.19 and 643.43 feet. During the May 2001 survey dates, the water surface fluctuated between elevation 649.05 and 649.06 feet. These elevations were provided by USGS gage 08045000 EAGLE MOUNTAIN RESERVOIR ABOVE FORTH WORTH, TX. After all corrections were applied to the raw data file, the edited file was saved with a different extension. The edited files were combined into a single (x,y,z) data file, to be used with the GIS software to develop a model of the lake's bottom surface.

The resulting data file was downloaded to a Sun Ultra 20 workstation running the UNIX operating system. Environmental System Research Institute's (ESRI) Arc/Info GIS software was used to convert the data to a MASS points file. The MASS points and the boundary file were then used to create a Digital Terrain Model (DTM) of the lake's bottom surface using Arc/Info's TIN software module. The module generates a triangulated irregular network (TIN) from the data points and the boundary file using a method known as Delauney's criteria for triangulation. A triangle is formed between three non-uniformly spaced points, including all points along the boundary. If there is another point within the triangle, additional triangles are created until all points lie on the vertex of a triangle. All of the data points are used in this method. The generated network of three-dimensional triangular planes represents the actual bottom surface. With this representation of the bottom, the software then calculates elevations along the triangle surface plane by determining the elevation along each leg of the triangle. The lake area and volume can be determined from the triangulated irregular network for water elevations of interest using this method of interpolation.

Volumes presented in Appendix A were calculated from the TIN using Arc/Info software. Surface areas presented in Appendix B were computed using Arc/Info software below elevation 649.1 feet.

Other products developed from the model include a shaded elevation range map (Figure 3) and a shaded depth range map (Figure 4). To develop these maps, the TIN was converted to a lattice using the TINLATTICE command and then to a polygon coverage using the LATTICEPOLY command. Linear filtration algorithms were applied to the DTM to produce smooth cartographic

contours. The resulting elevation contour map of the bottom surface at two-foot intervals is presented in Figure 5. Cross sections for the range lines shown on Figures 3, 4, and 5 are presented Appendix G.

RESULTS

Results from the 2000 TWDB survey indicate that Eagle Mountain Lake encompasses 8702 surface acres and contains a total volume of 182,505 acre-feet at pool elevation 649.1 feet. Dead pool storage, the volume below the invert elevation of the low-flow outlet pipe at 599.9 feet, is 5 acre-feet. Thus, the conservation storage at elevation 649.1 (total volume - dead storage) for Eagle Mountain Lake is 182,500 acre-feet. The shoreline at pool elevation 649.1 feet was calculated to be approximately 83.5 miles. The deepest point of the lake measured during the surveys was at elevation 598.2 feet and corresponds to a depth of 50.9 feet relative to conservation pool elevation 649.1 feet. That depth was located approximately 1,200 feet upstream from the center of Eagle Mountain Dam.

SUMMARY AND COMPARISONS

Eagle Mountain Lake was initially impounded in 1934. Storage calculations in 1988 (Freese and Nichols, 1988) reported the volume at pool elevation 649.1 feet to be 178,440 acre-feet with a surface area of 9,030 acres.

TWDB staff performed a volumetric survey of Eagle Mountain Lake between April 18 and April 26, 2000. Subsequent data were collected on May 16 and May 17, 2001 when the lake elevation had risen to near conservation pool elevation. The 2000-2001 survey utilized differential global positioning system, depth sounder and geographical information system technology to create a digital model of the lake's bathymetry. Results indicate that the lake's volume at the pool elevation of 649.1 feet is 182,505 acre-feet, with a corresponding area of 8,702 acres.

Comparisons between the 1968, 1988 and present (2000-2001) surveys are described in Table

1 for elevation 649.1 feet msl. Decreases in volume and area were found between 1968 and 1988, while volume increased and area decreased between 1988 and 2000. The increase in volume between 1988 and 2000 is probably due to differences in technology used during the surveys. Between 1968 and 2000, the area decreased by 498 acres (-5.7%), and the volume decreased by 7,955 (-4.2 %) acre-feet. Comparing between the 1988 and 2000 results, the area decreased by 328 acres (-3.6%), and the volume increased by 4,065 acre-feet (+2.3 %). The difference in volume between 1968 and 2000 (volume measured in 2000 minus volume measured in 1968) is presented in Appendix E at elevations between 599 feet and 649 feet msl. A similar plot for difference in area (area measured in 2000 minus area measured in 1968) is presented in Appendix F. The loss in area shown in Appendix F between elevations 599 feet and approximately 616 feet msl and again between elevations of approximately 623 feet and 649 feet suggests that sedimentation occurs primarily between these elevations. The increase in area between elevations of approximately 616 feet and 623 feet msl suggests that some erosion takes place at those elevations. Again, comparison between the data sets is difficult and some changes might simply be due to methodological differences.

It is recommended that the same methodology be used in five to ten years or after major flood events to monitor changes to the lake's storage volume, and that a survey be conducted when the lake again reaches conservation pool elevation to allow calculation of volumes and areas up to conservation pool elevation.

Table 1. Area and volume comparisons at elevation 649.1 feet msl.

Year	1968	1988	2000
Area (acres)	9,200	9,030	8,702
Volume (acre-feet)	190,460	178,440	182,505

REFERENCES

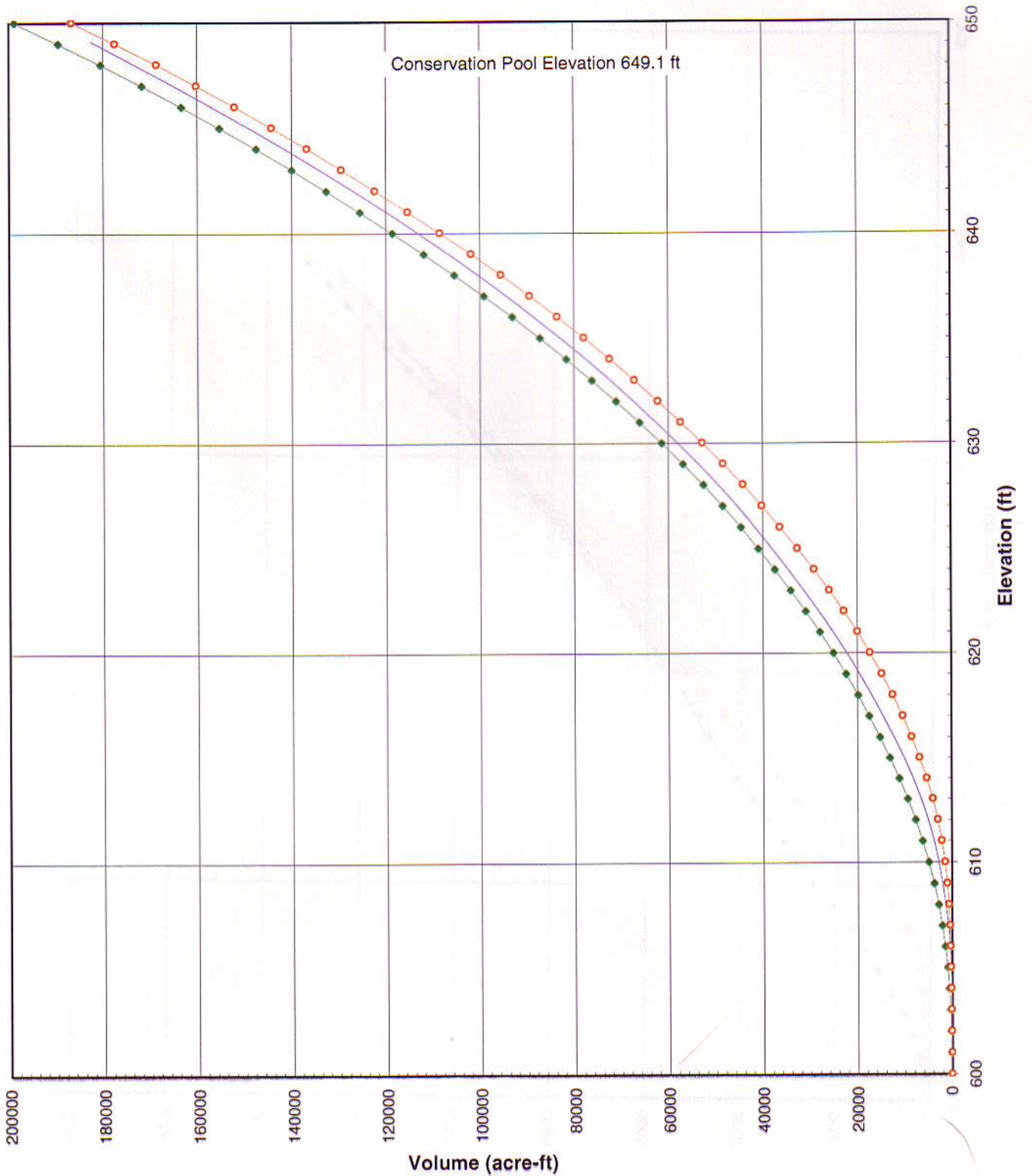
1. Texas Water Development Board. 1966. Dams and Lakes in Texas, Historical and Descriptive

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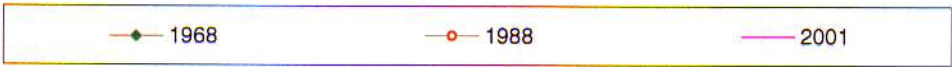
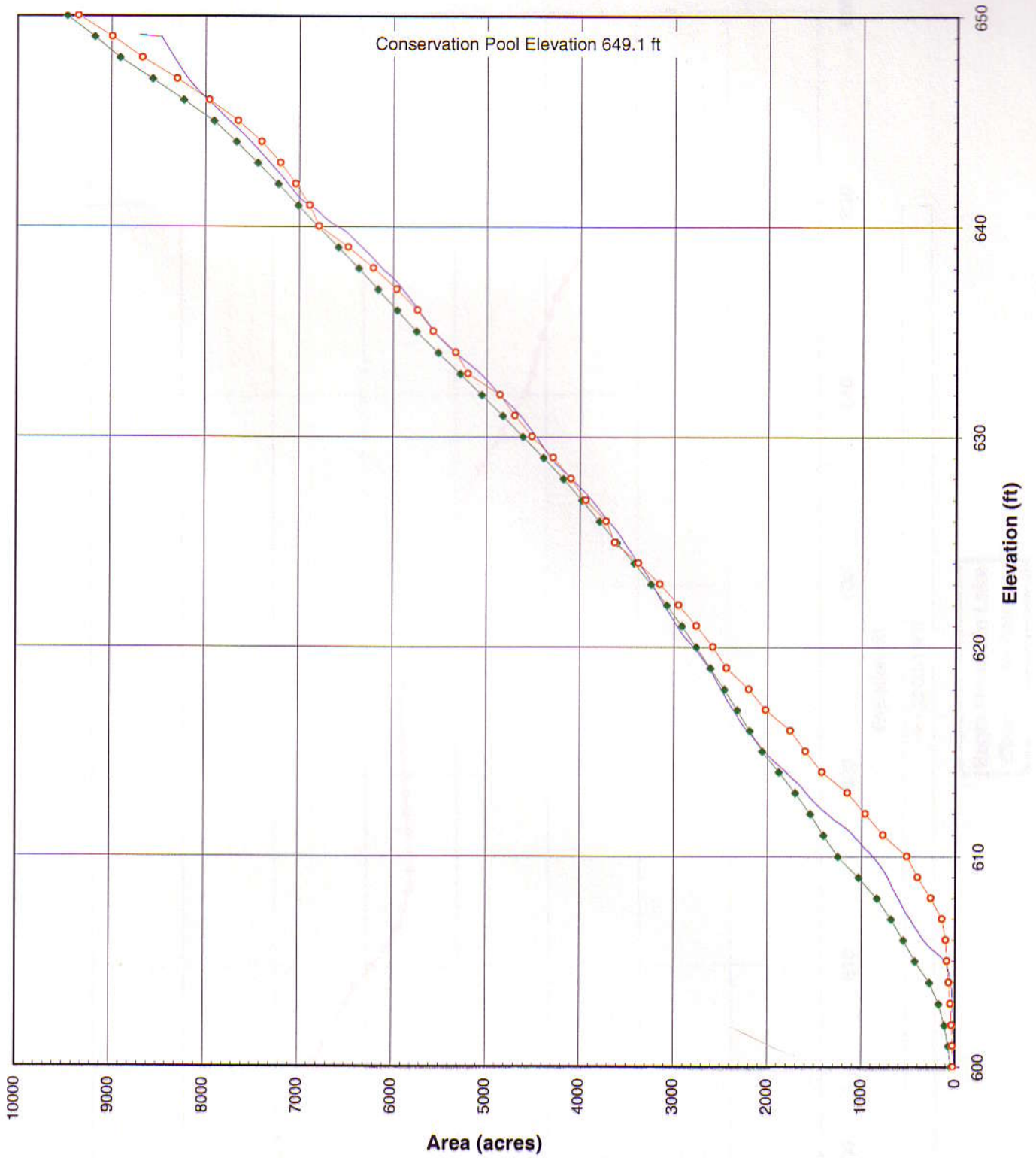
2. Texas Water Development Board. 1974. Engineering Data on Dams and Lakes in Texas. Part I. Report 126.

3. Freese and Nichols. 1988. Lake Bridgeport / Eagle Mountain Lake Sedimentation Survey.

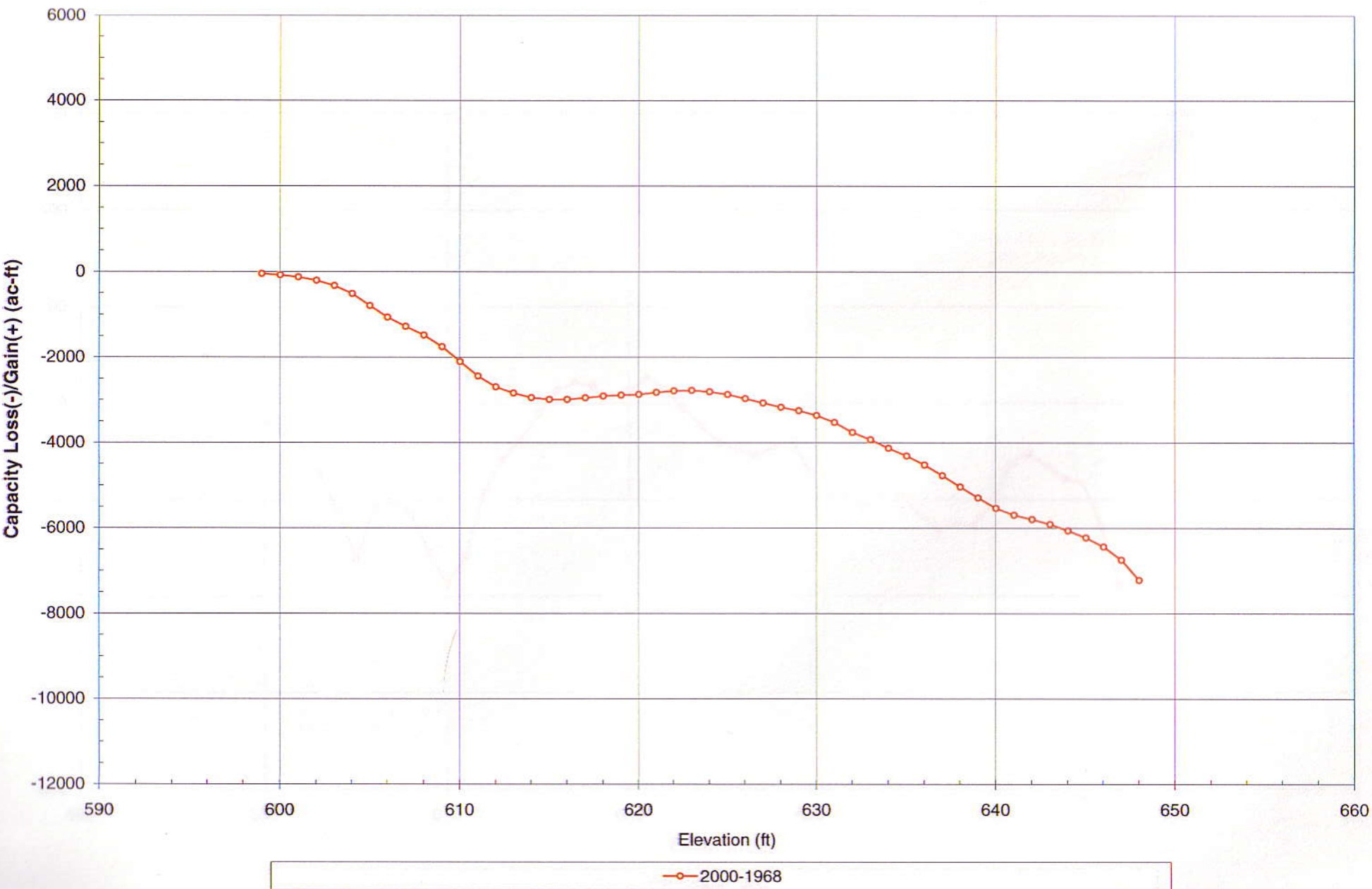
4. United States Geological Survey. 1998. Water Resources Data – Texas. Volume 1.



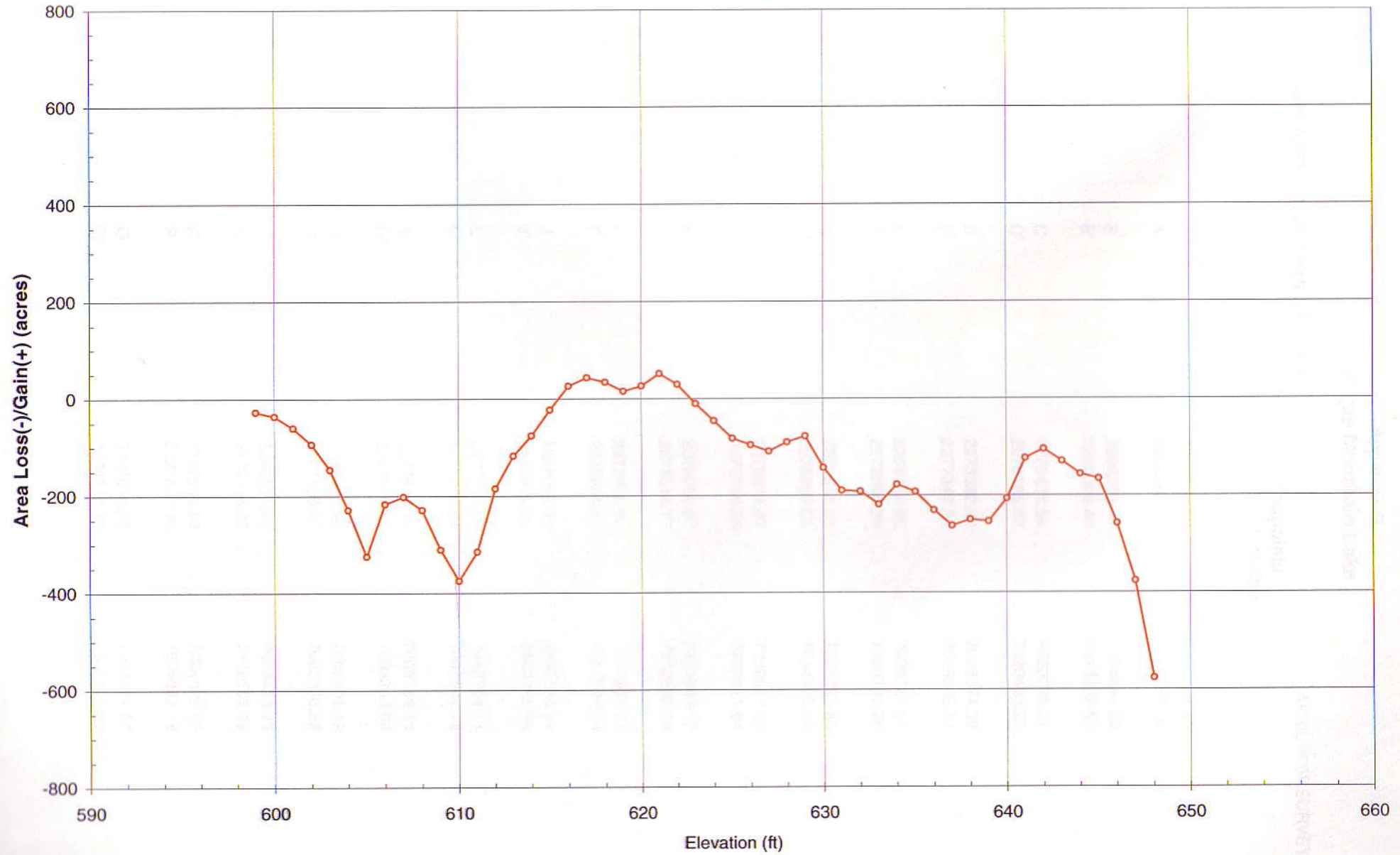
Eagle Mountain Lake
 May 2001
 Volume vs Elevation



Eagle Mountain Lake
 May 2001
 Area vs Elevation



**Eagle Mountain Lake
Changes in Volume**



—○— 2000-1968

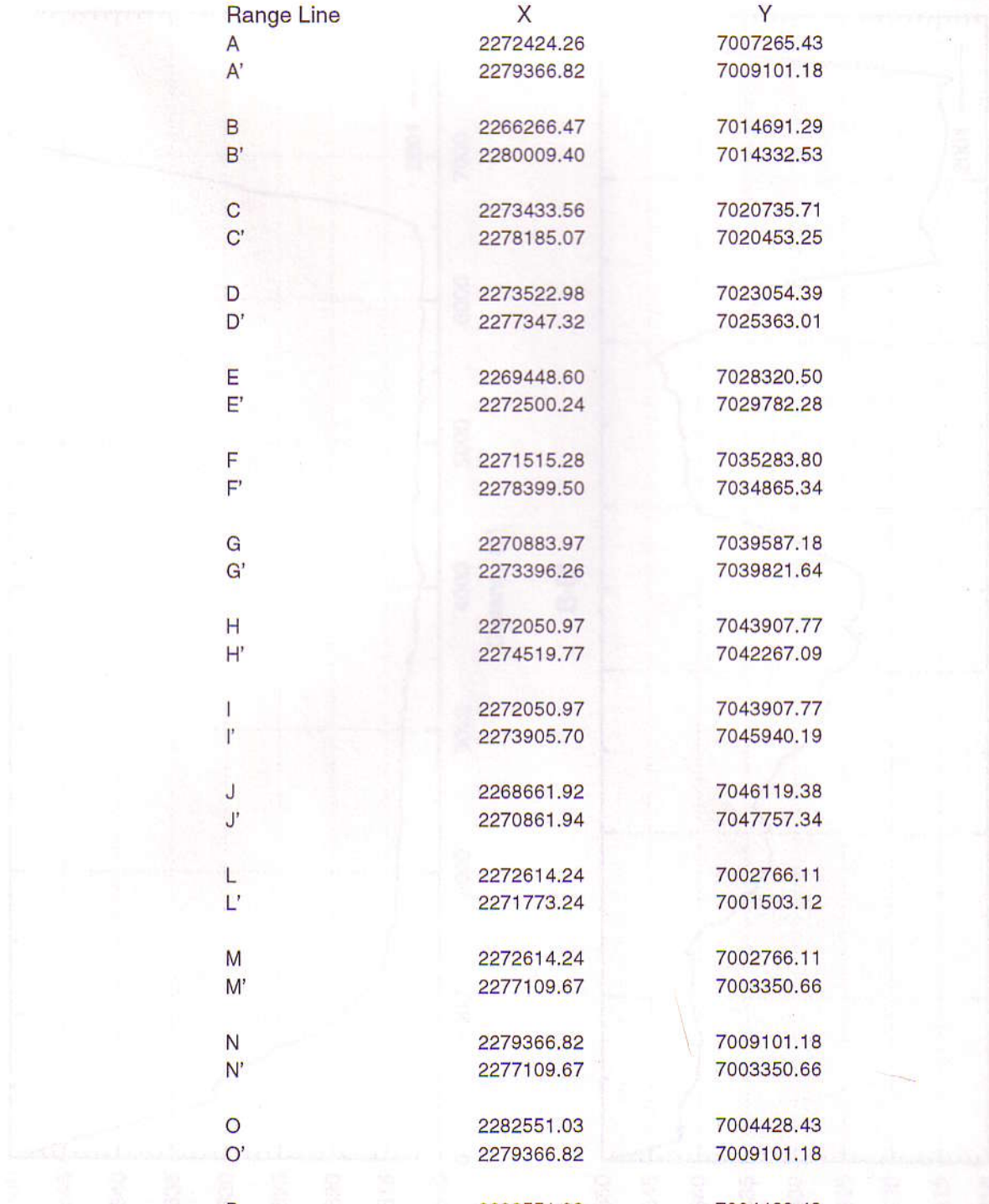
**Eagle Mountain Lake
Changes in Area**

Appendix G
Eagle Mountain Lake

TEXAS WATER DEVELOPMENT BOARD

APRIL 2000 SURVEY

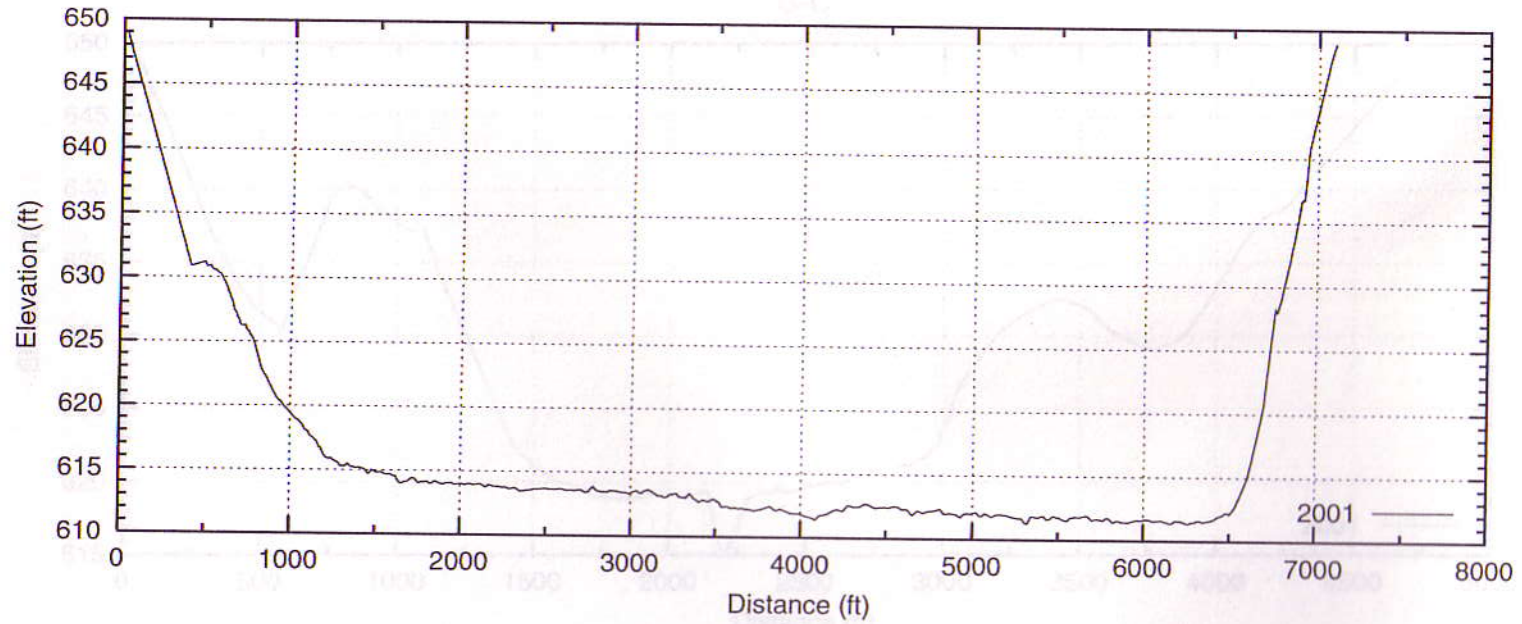
Range Line Endpoints
State Plane NAD83 Units-feet



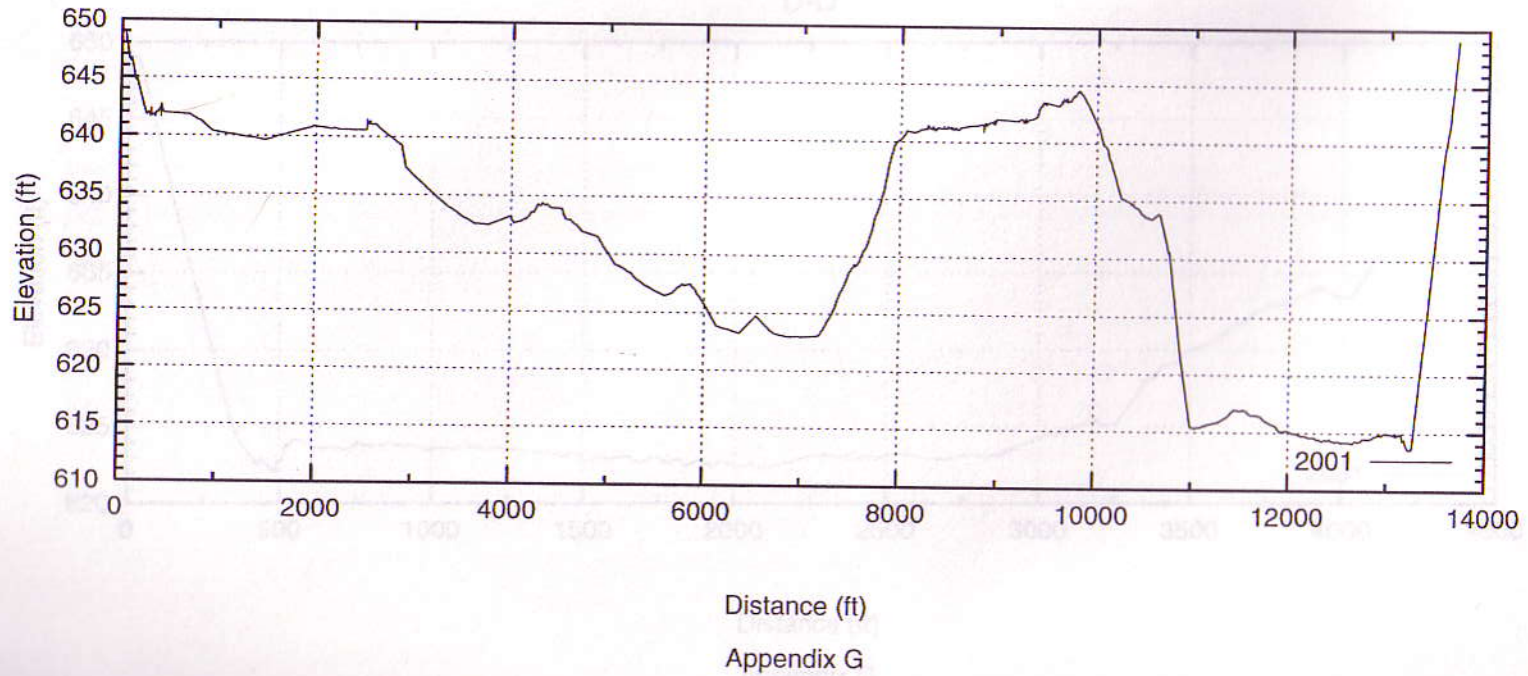
Range Line	X	Y
A	2272424.26	7007265.43
A'	2279366.82	7009101.18
B	2266266.47	7014691.29
B'	2280009.40	7014332.53
C	2273433.56	7020735.71
C'	2278185.07	7020453.25
D	2273522.98	7023054.39
D'	2277347.32	7025363.01
E	2269448.60	7028320.50
E'	2272500.24	7029782.28
F	2271515.28	7035283.80
F'	2278399.50	7034865.34
G	2270883.97	7039587.18
G'	2273396.26	7039821.64
H	2272050.97	7043907.77
H'	2274519.77	7042267.09
I	2272050.97	7043907.77
I'	2273905.70	7045940.19
J	2268661.92	7046119.38
J'	2270861.94	7047757.34
L	2272614.24	7002766.11
L'	2271773.24	7001503.12
M	2272614.24	7002766.11
M'	2277109.67	7003350.66
N	2279366.82	7009101.18
N'	2277109.67	7003350.66
O	2282551.03	7004428.43
O'	2279366.82	7009101.18
P	2282551.03	7004428.43
P'	2286697.48	7006692.76
Q	2288015.97	7006852.57
Q'	2288326.72	7003239.56

Eagle Mountain Lake

A-A'



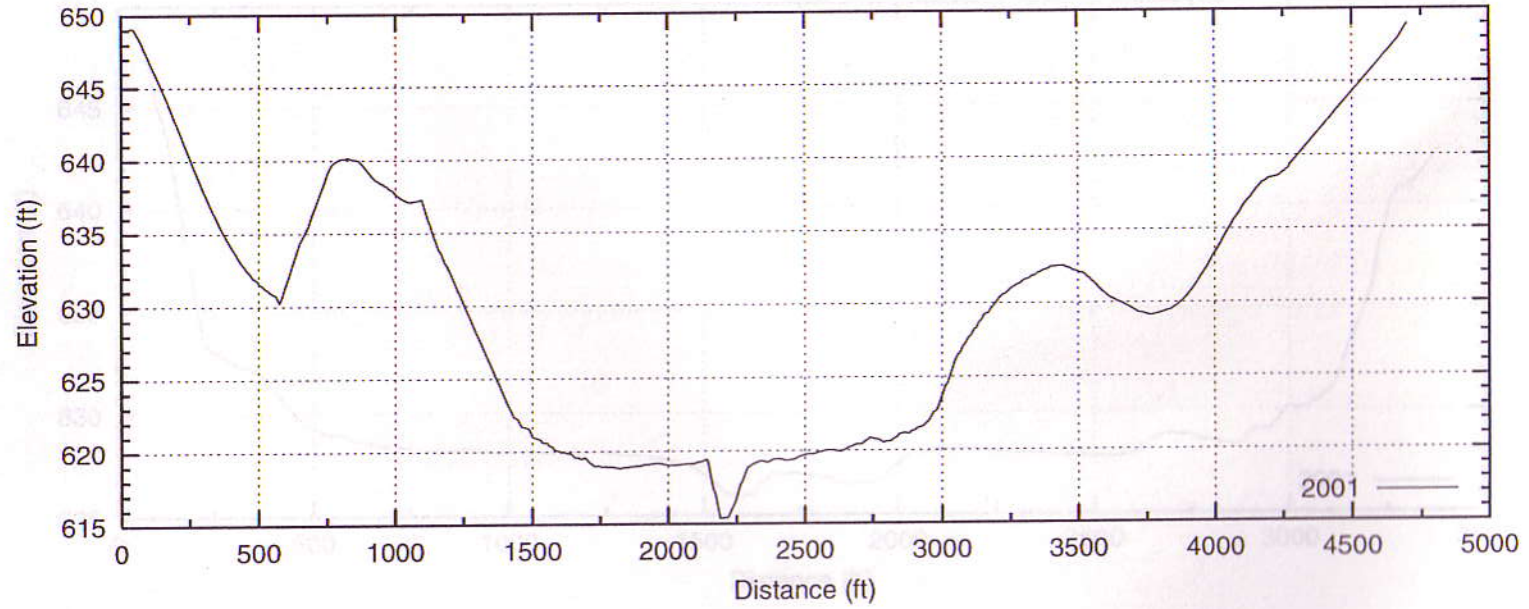
B-B'



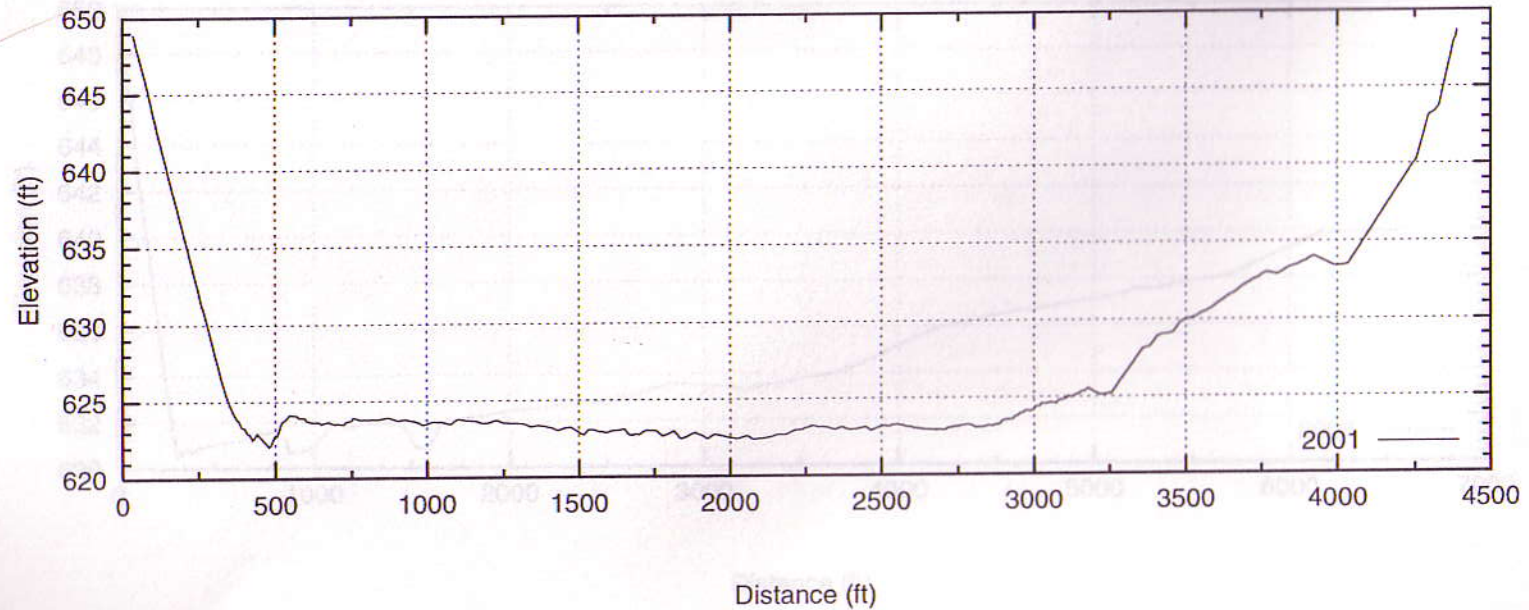
Distance (ft)

Appendix G

Eagle Mountain Lake C-C'



D-D'

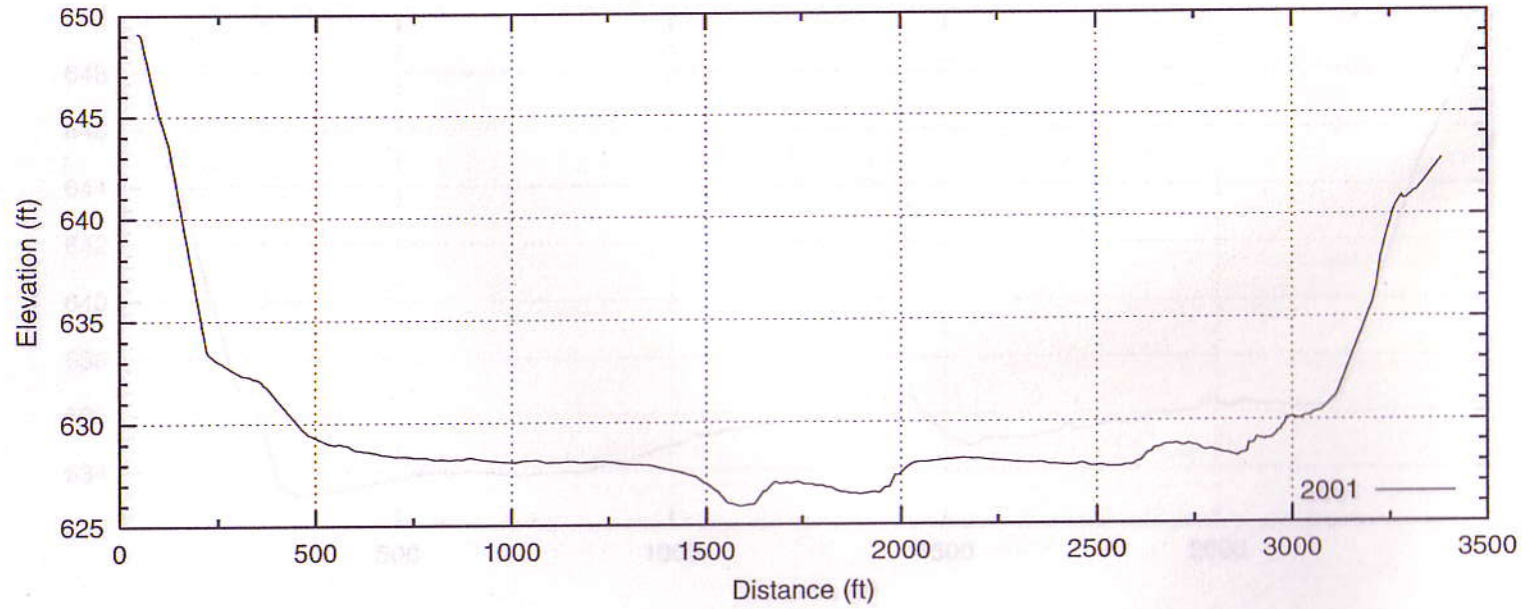


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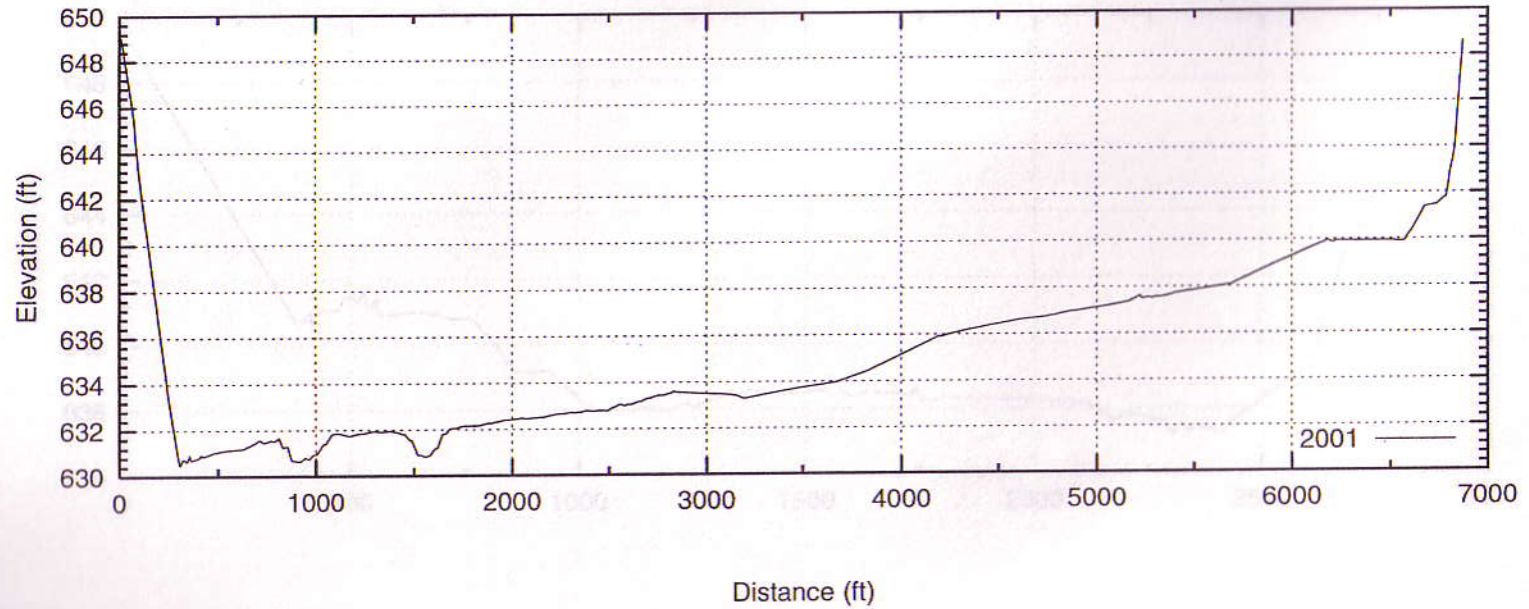
Appendix G

Eagle Mountain Lake

E-E'



F-F'

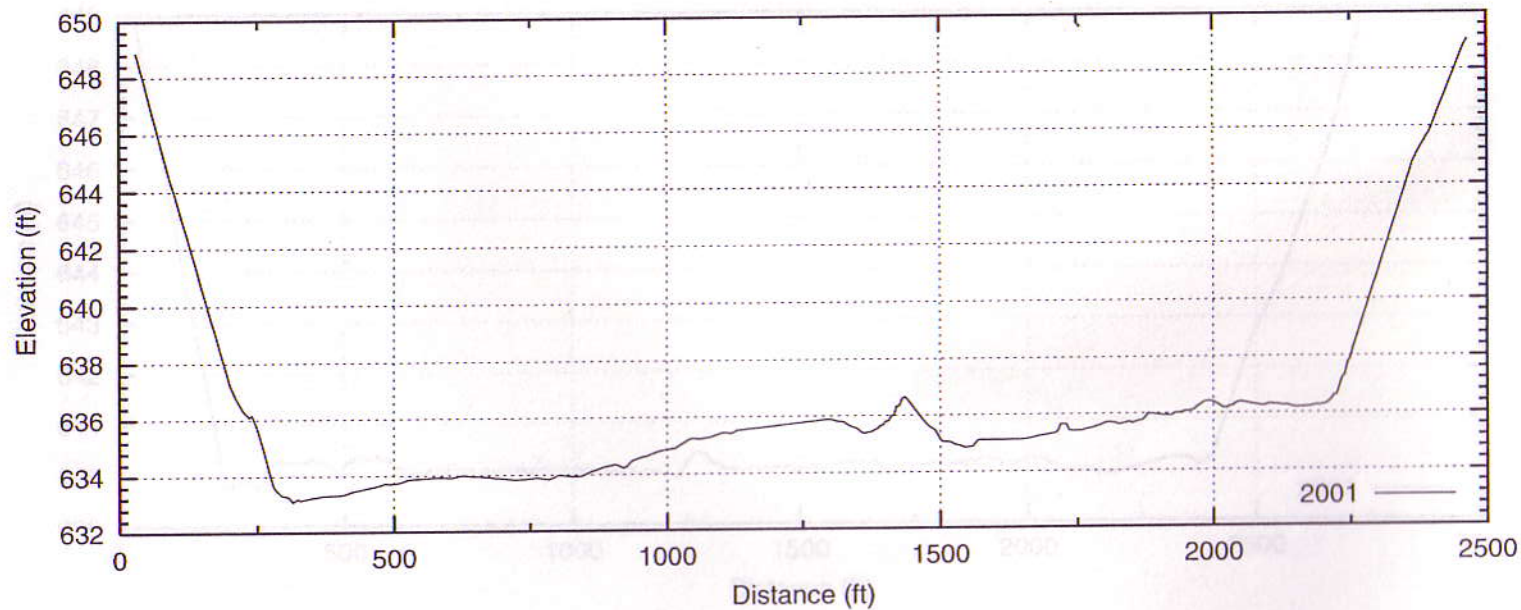


Distance (ft)

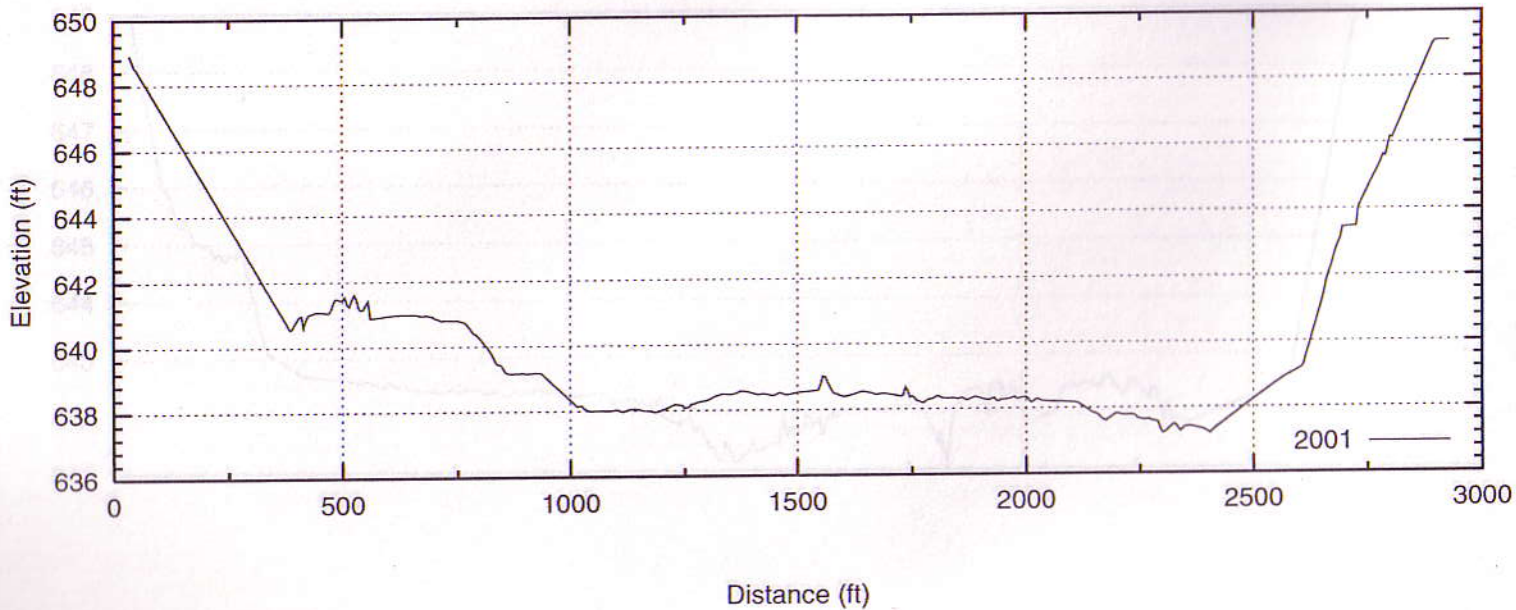
Appendix G

Eagle Mountain Lake

G-G'

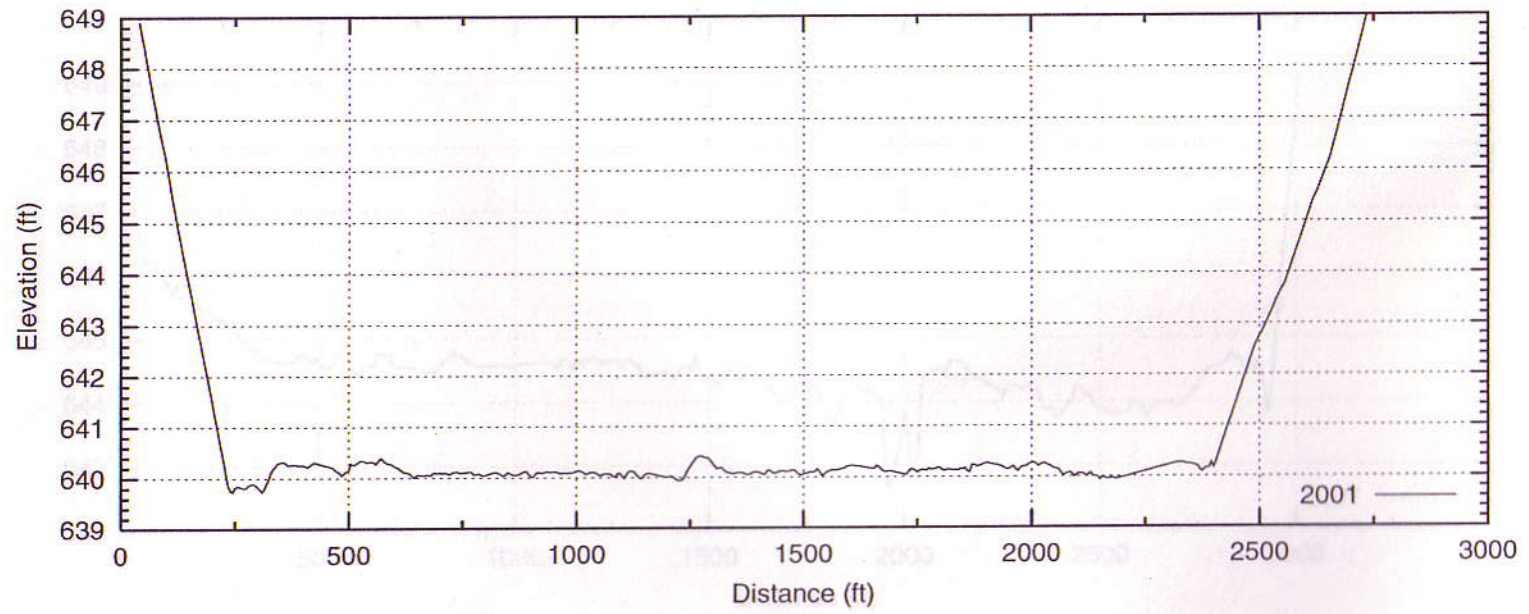


H-H'

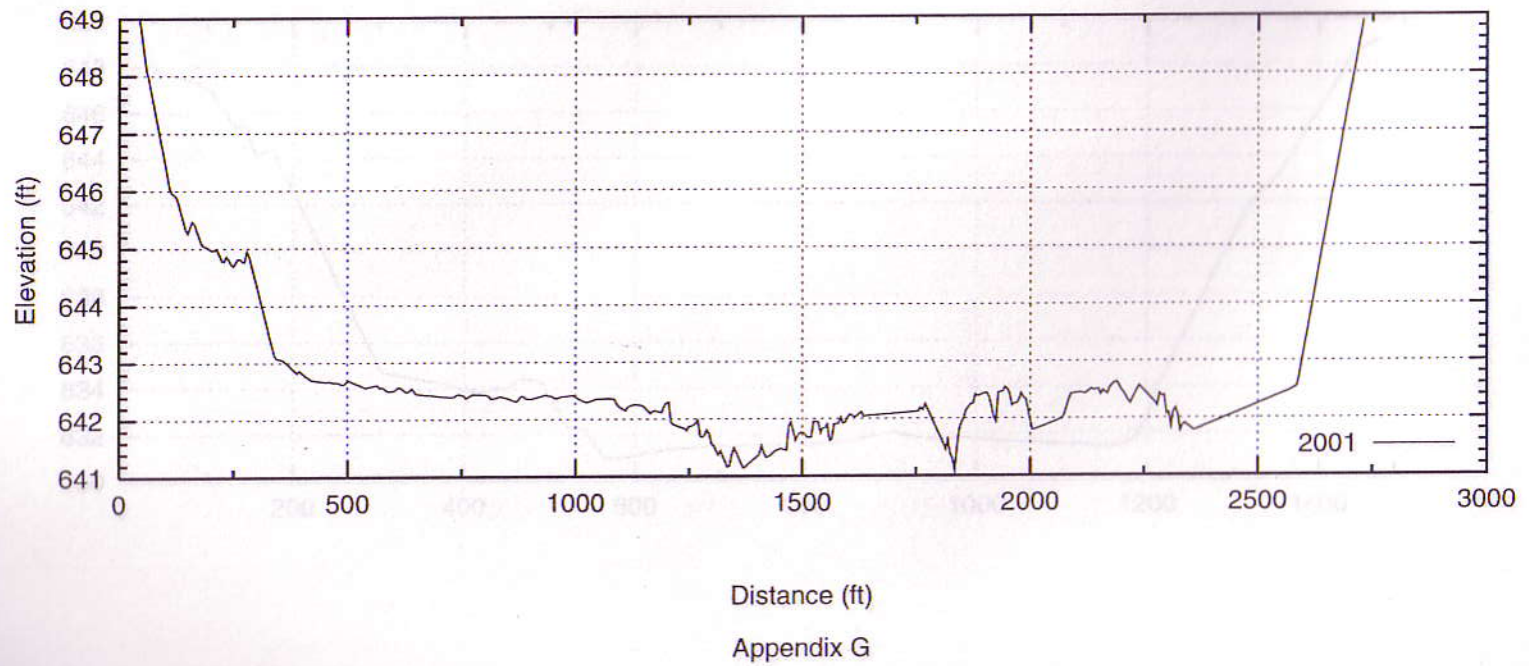


Eagle Mountain Lake

I-I'



J-J'

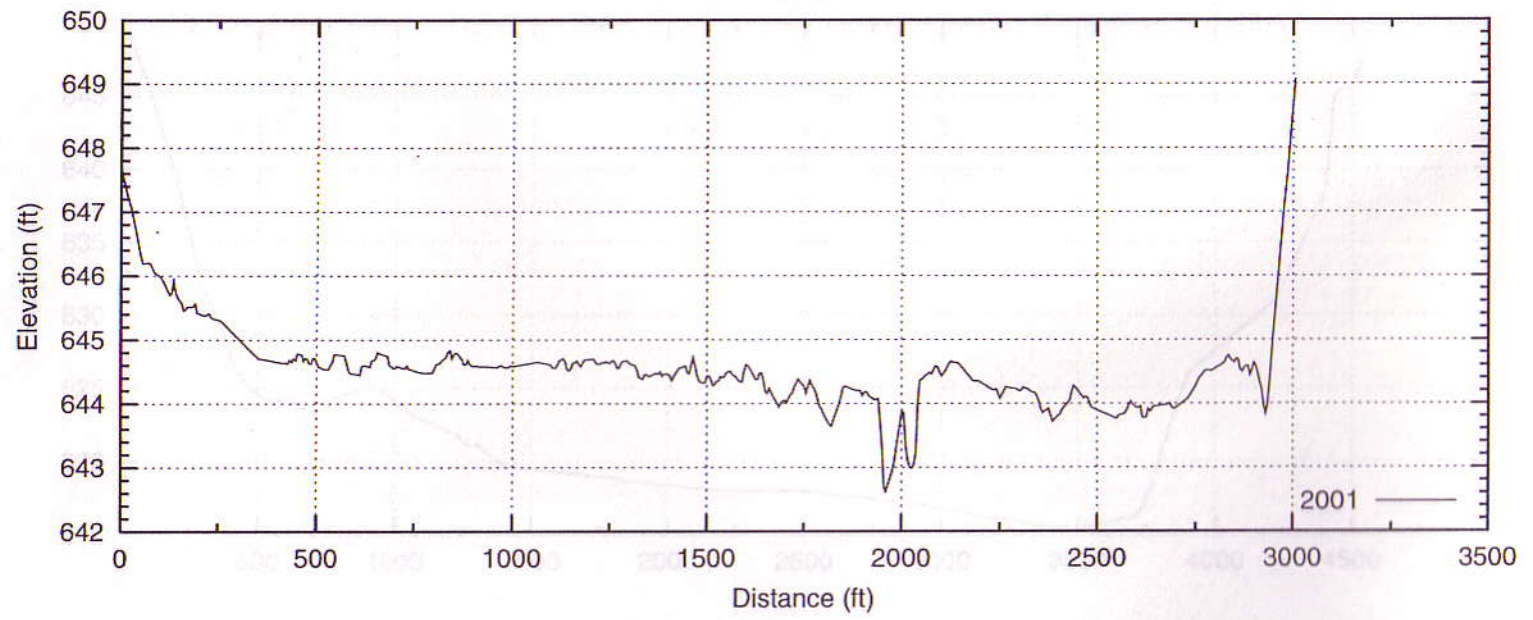


Distance (ft)

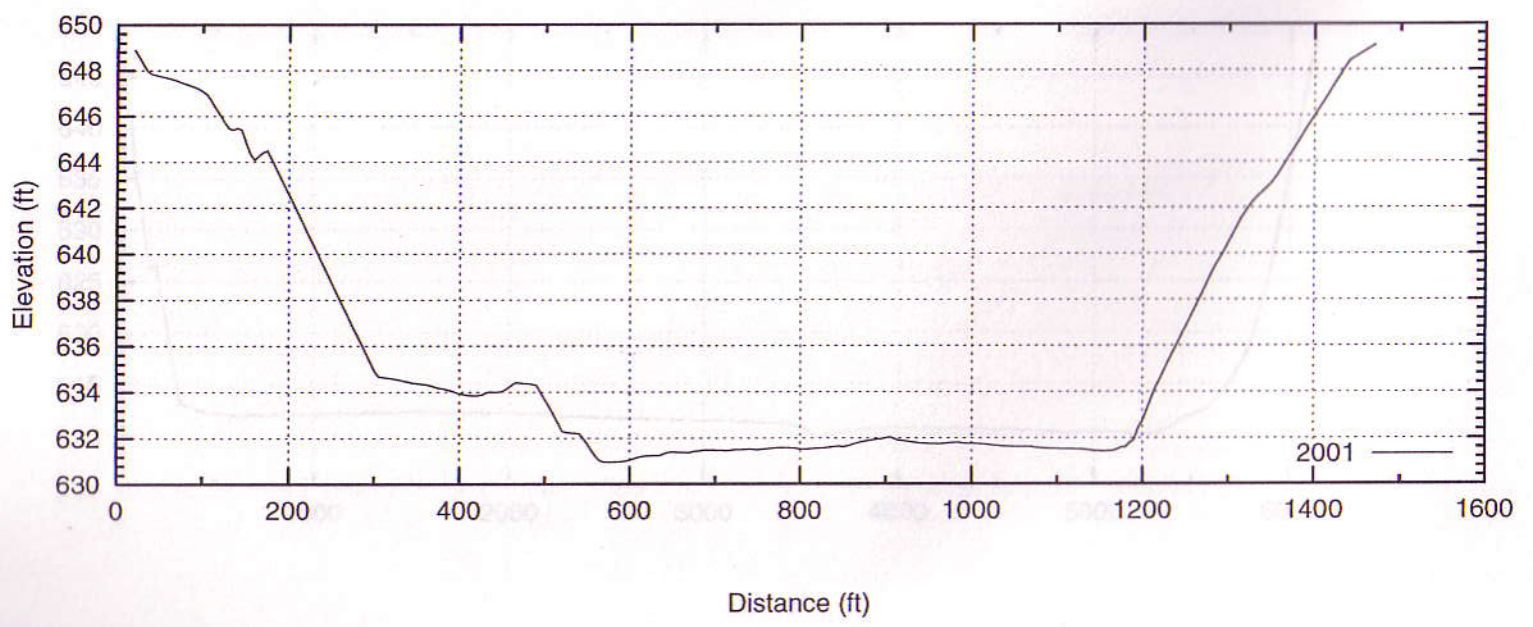
Appendix G

Eagle Mountain Lake

K-K'

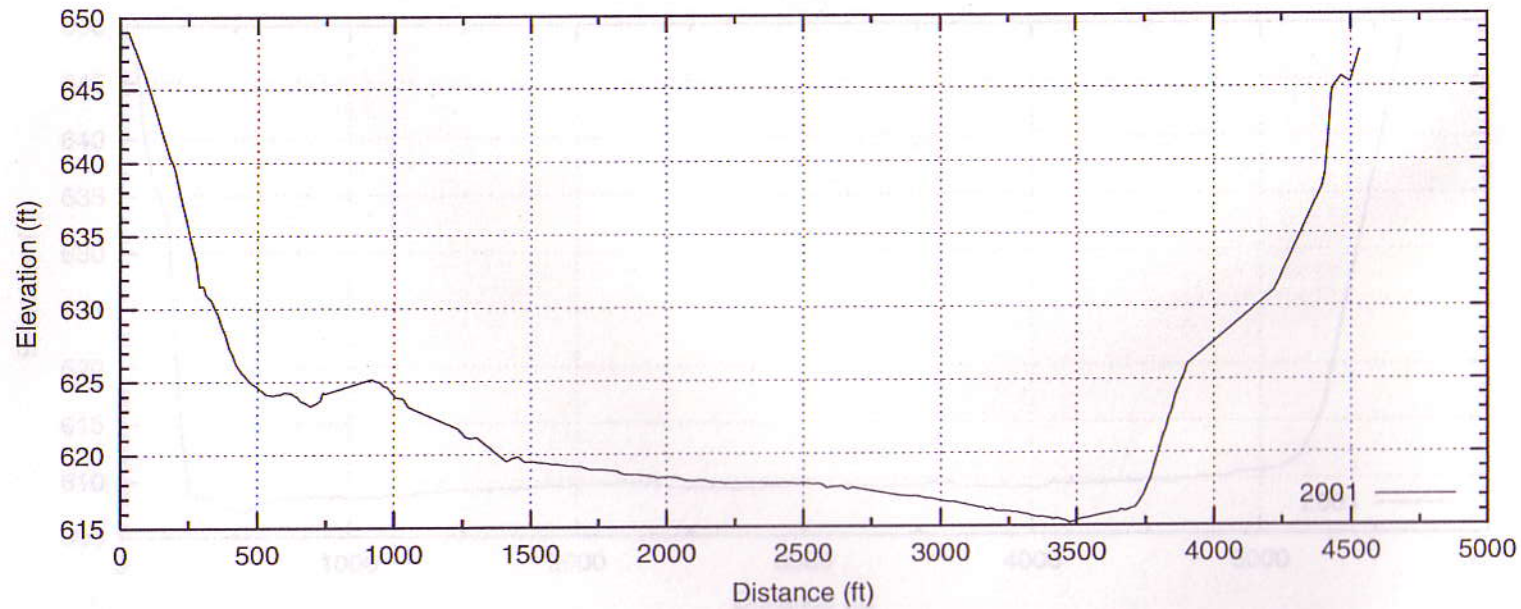


L-L'

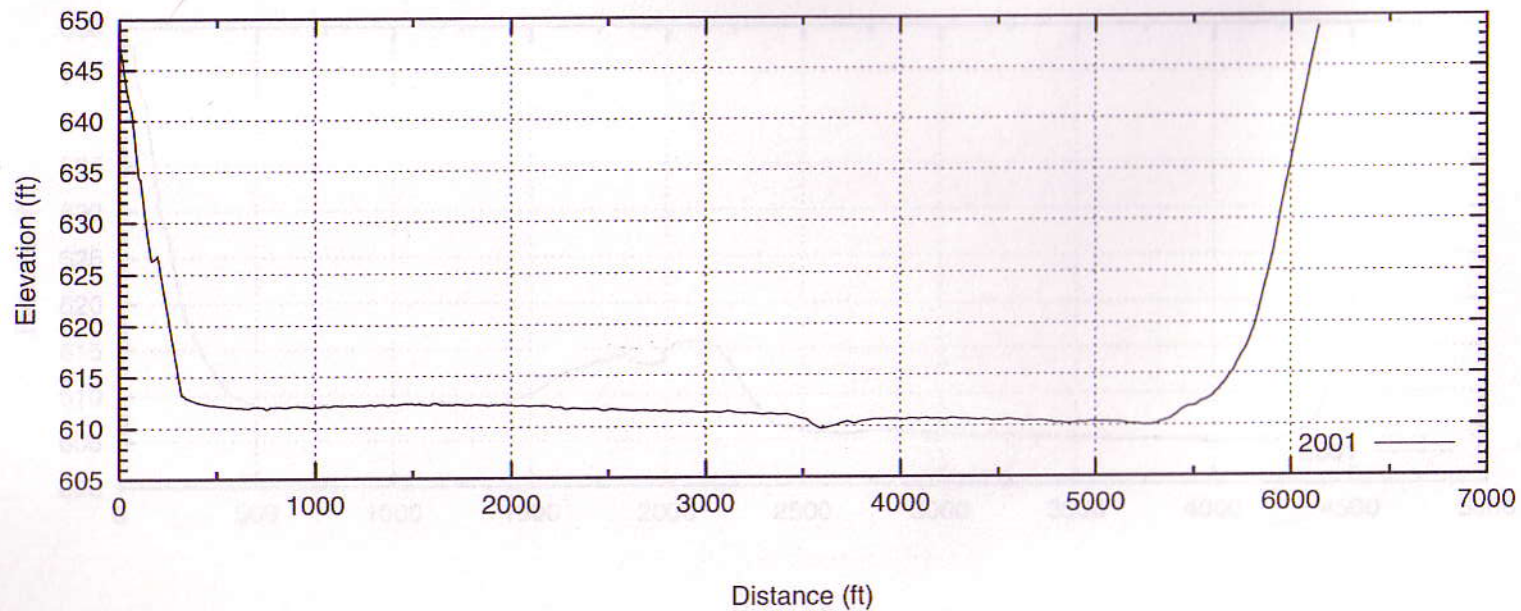


Eagle Mountain Lake

M-M'



N-N'

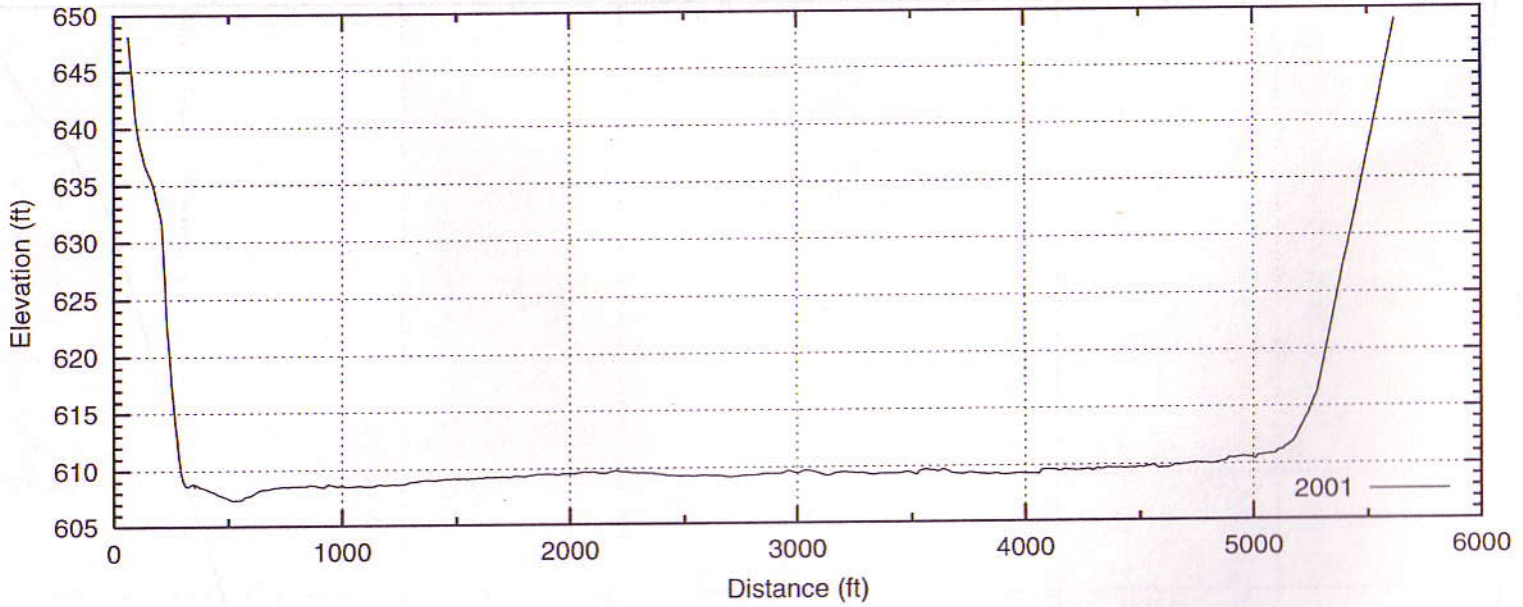


Distance (ft)

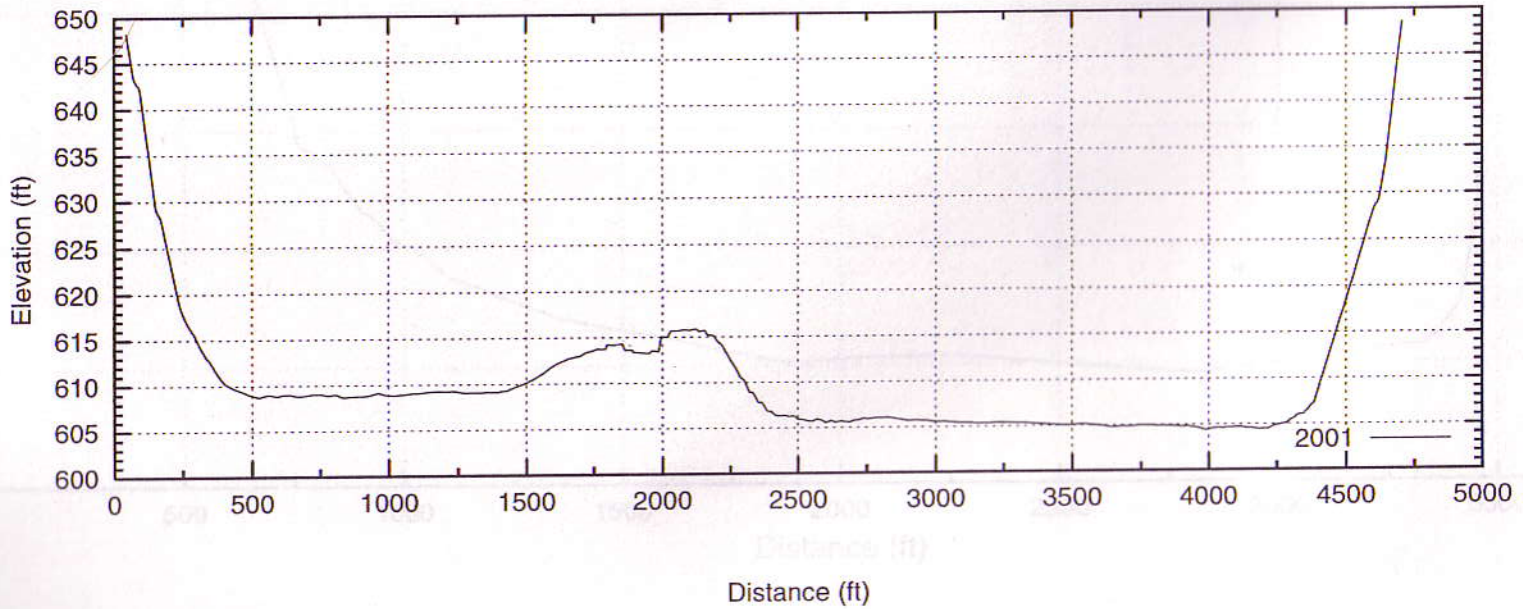
Appendix G

Eagle Mountain Lake

O-O'

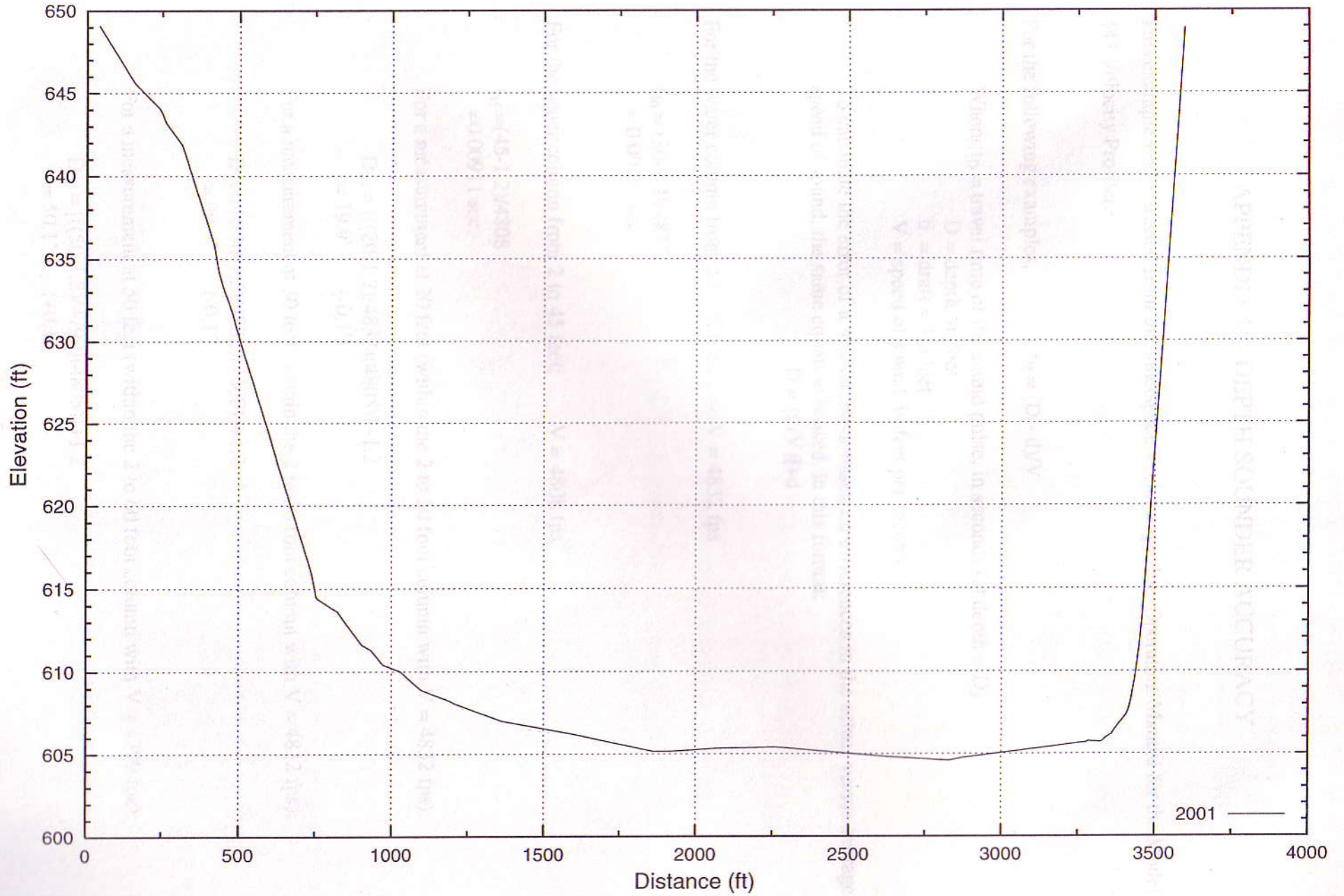


P-P'



Eagle Mountain Lake

Cross Section Q-Q'



2001

APPENDIX H - DEPTH SOUNDER ACCURACY

This example was extracted from the Innerspace Technology, Inc. Operation Manual for the Model 443 Velocity Profiler.

For the following examples, $t_D = (D - d)/V$

Where: t_D = travel time of the sound pulse, in seconds (at depth = D)

D = depth, in feet

d = draft = 1.2 feet

V = speed of sound, in feet per second

To calculate the error of a measurement based on differences in the actual versus average speed of sound, the same equation is used, in this format:

$$D = [t (V)]+d$$

For the water column from 2 to 30 feet: $V = 4832$ fps

$$\begin{aligned} t_{30} &= (30-1.2)/4832 \\ &= 0.00596 \text{ sec.} \end{aligned}$$

For the water column from 2 to 45 feet: $V = 4808$ fps

$$\begin{aligned} t_{45} &= (45-1.2)/4808 \\ &= 0.00911 \text{ sec.} \end{aligned}$$

For a measurement at 20 feet (within the 2 to 30 foot column with $V = 4832$ fps):

$$\begin{aligned} D_{20} &= [((20-1.2)/4832)(4808)]+1.2 \\ &= 19.9' \quad (-0.1') \end{aligned}$$

For a measurement at 30 feet (within the 2 to 30 foot column with $V = 4832$ fps):

$$\begin{aligned} D_{30} &= [((30-1.2)/4832)(4808)]+1.2 \\ &= 29.9' \quad (-0.1') \end{aligned}$$

For a measurement at 50 feet (within the 2 to 60 foot column with $V = 4799$ fps):

$$D_{50} = [((50-1.2)/4799)(4808)]+1.2$$

$$= 50.1' \quad (+0.1')$$

For the water column from 2 to 60 feet: $V = 4799$ fps Assumed $V_{80} = 4785$ fps

$$t_{60} = (60-1.2)/4799 \\ = 0.01225 \text{ sec.}$$

For a measurement at 10 feet (within the 2 to 30 foot column with $V = 4832$ fps):

$$D_{10} = [((10-1.2)/4832)(4799)]+1.2 \\ = 9.9' \quad (-0.1')$$

For a measurement at 30 feet (within the 2 to 30 foot column with $V = 4832$ fps):

$$D_{30} = [((30-1.2)/4832)(4799)]+1.2 \\ = 29.8' \quad (-0.2')$$

For a measurement at 45 feet (within the 2 to 45 foot column with $V = 4808$ fps):

$$D_{45} = [((45-1.2)/4808)(4799)]+1.2 \\ = 44.9' \quad (-0.1')$$

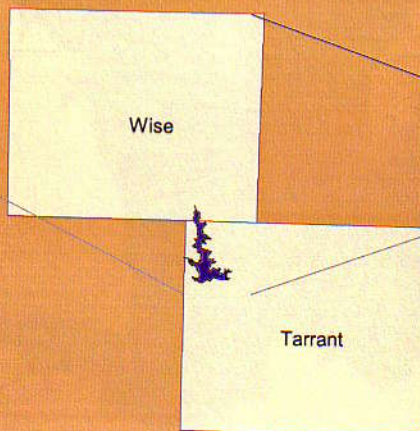
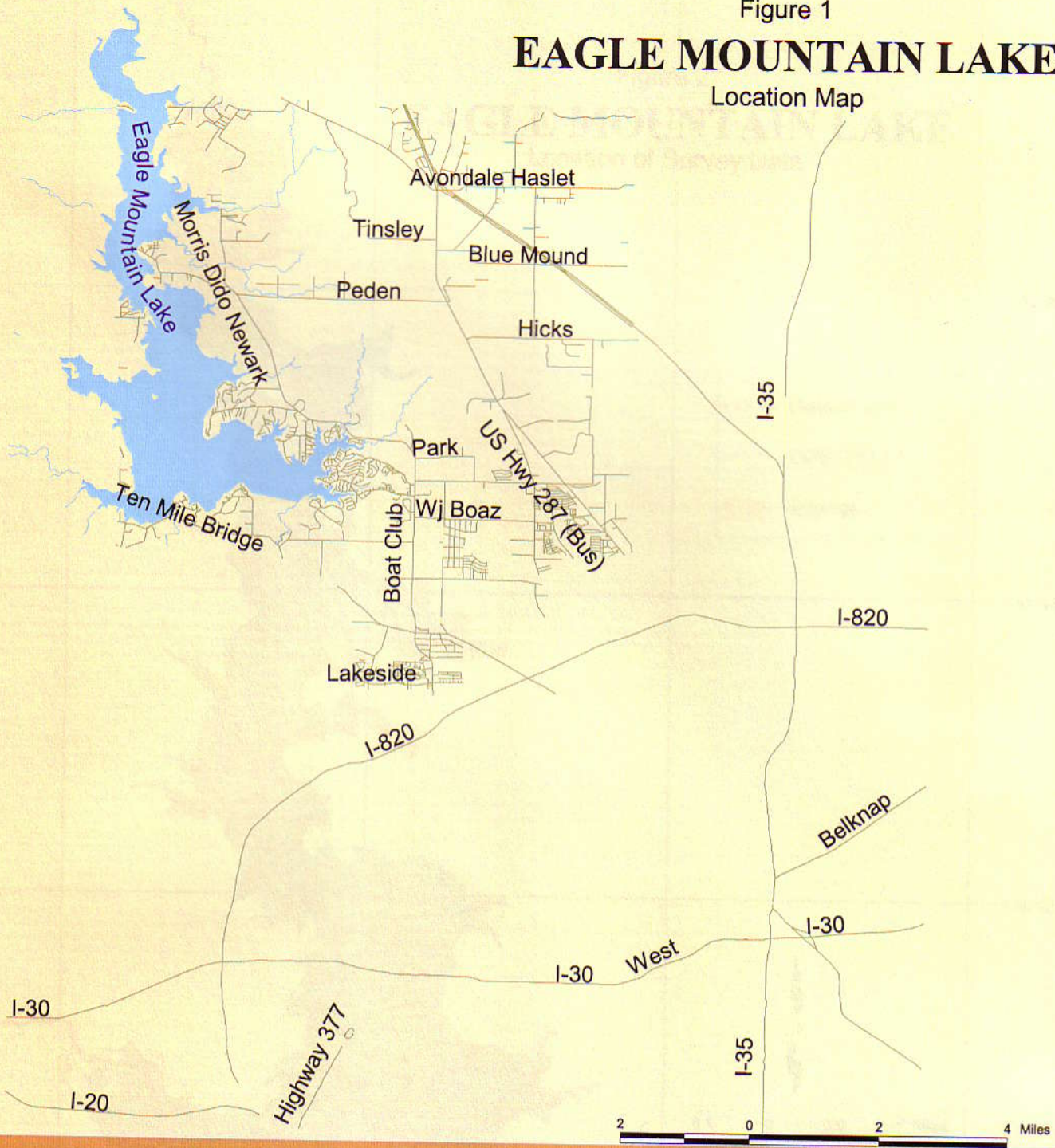
For a measurement at 80 feet (outside the 2 to 60 foot column, assumed $V = 4785$ fps):

$$D_{80} = [((80-1.2)/4785)(4799)]+1.2 \\ = 80.2' \quad (+0.2')$$

Figure 1

EAGLE MOUNTAIN LAKE

Location Map



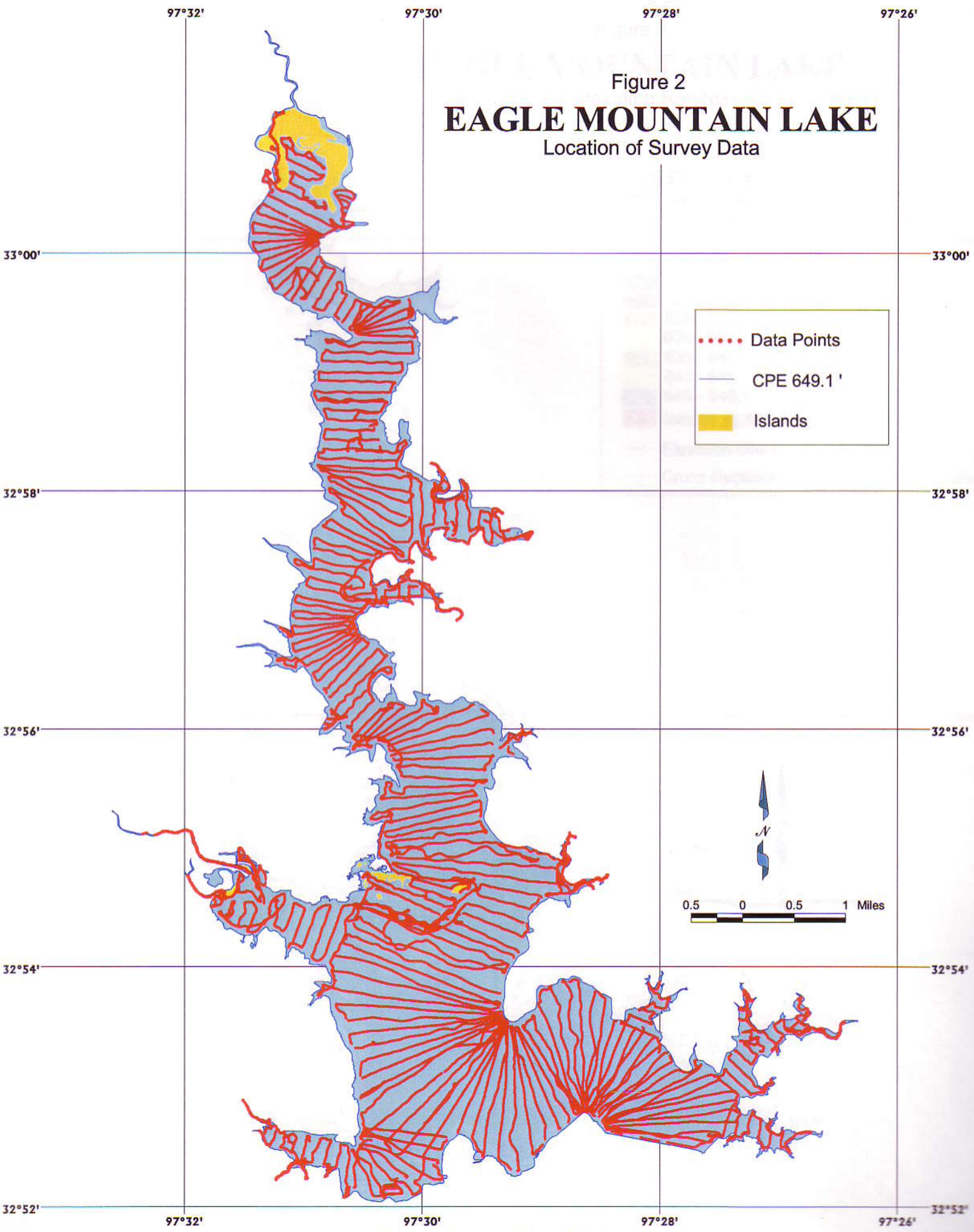


Figure 2
EAGLE MOUNTAIN LAKE
Location of Survey Data

97°32'

97°30'

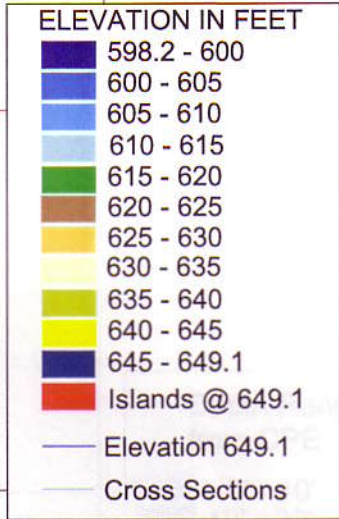
97°28'

97°26'

Figure 3

EAGLE MOUNTAIN LAKE

Elevation Ranges



33°00'

33°00'

32°58'

32°58'

32°56'

32°56'

32°54'

32°54'

32°52'

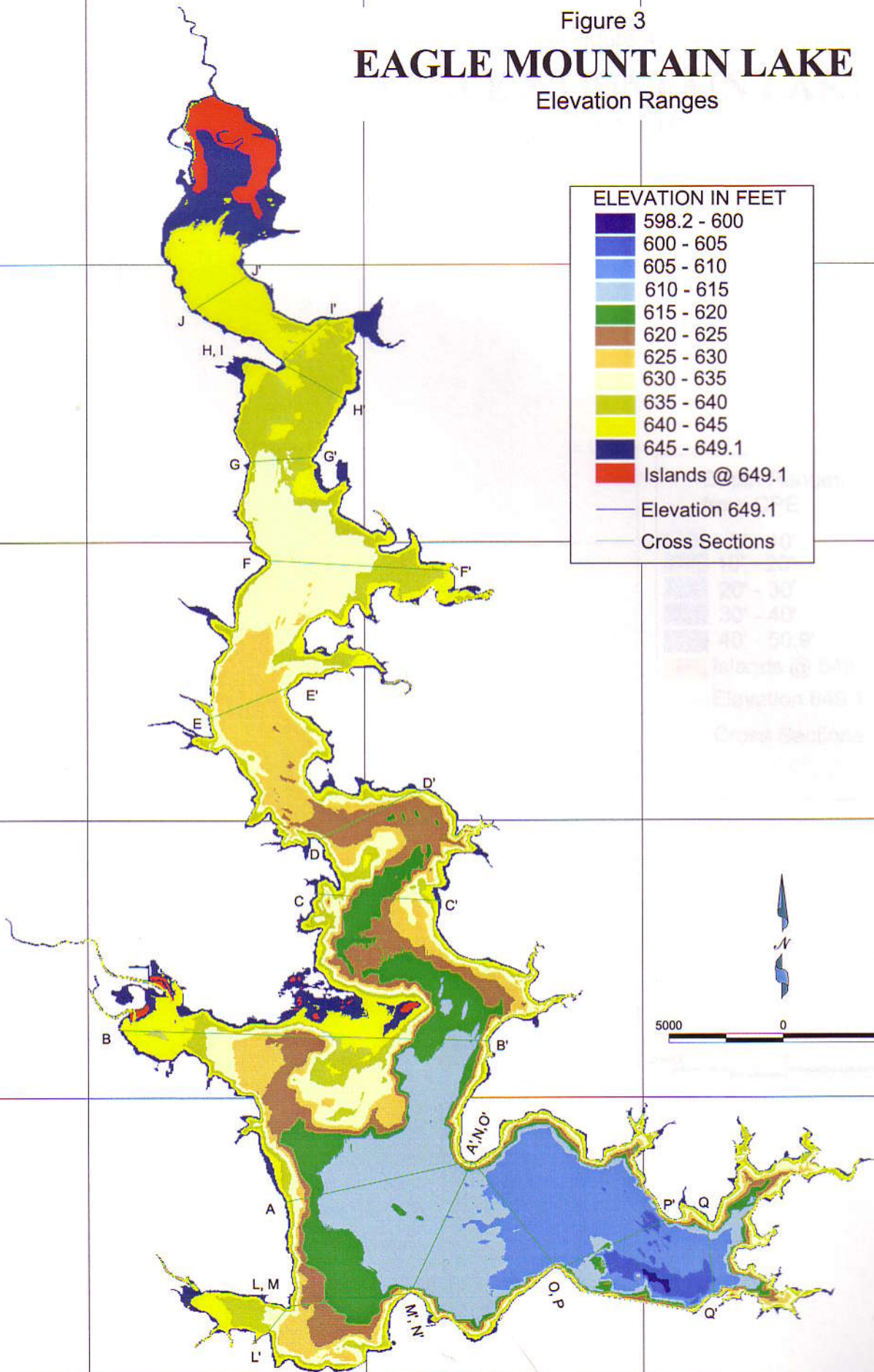
32°52'

97°32'

97°30'

97°28'

97°26'



97°32'

97°30'

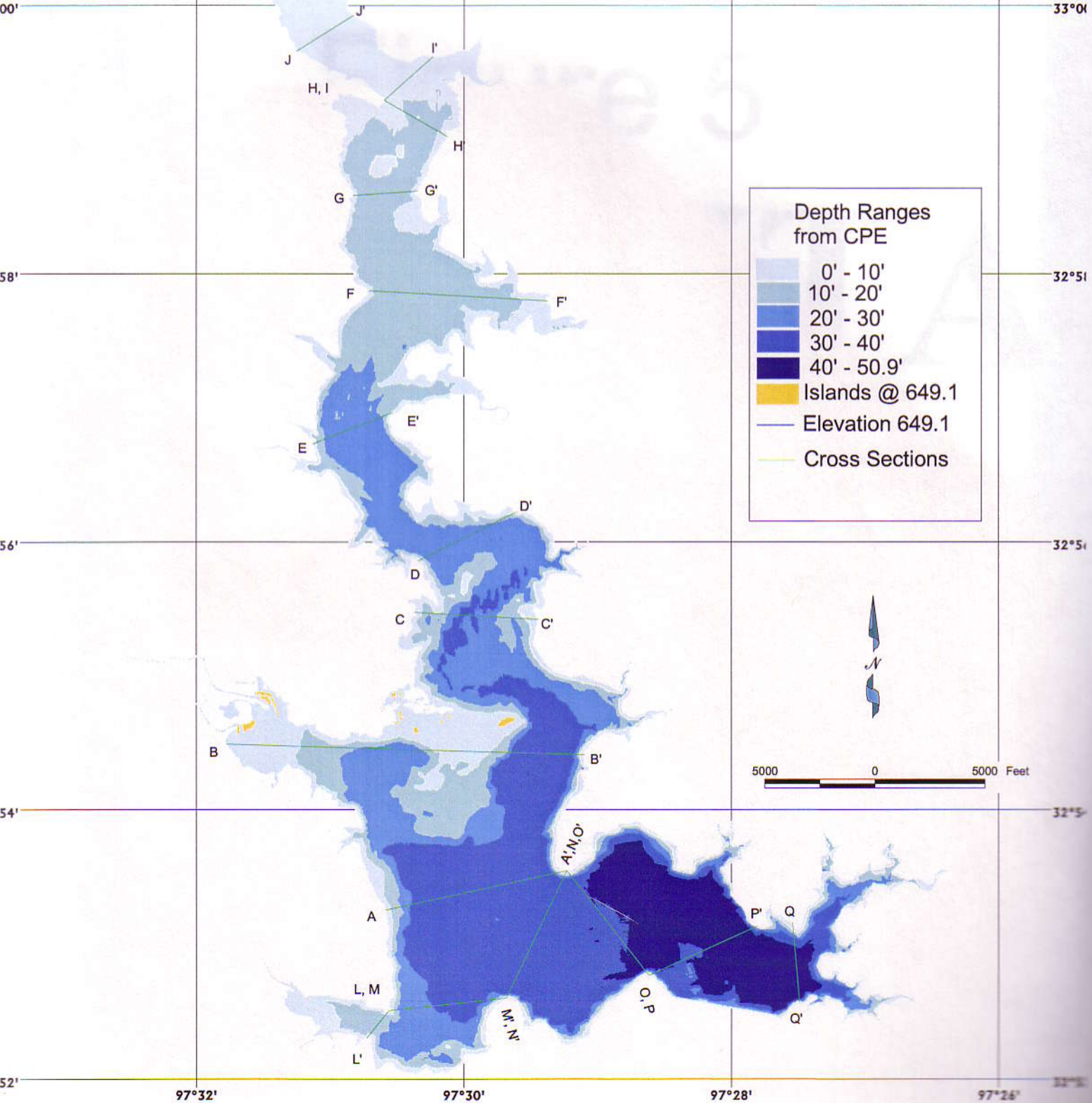
97°28'

97°26'

Figure 4

EAGLE MOUNTAIN LAKE

Depth Ranges



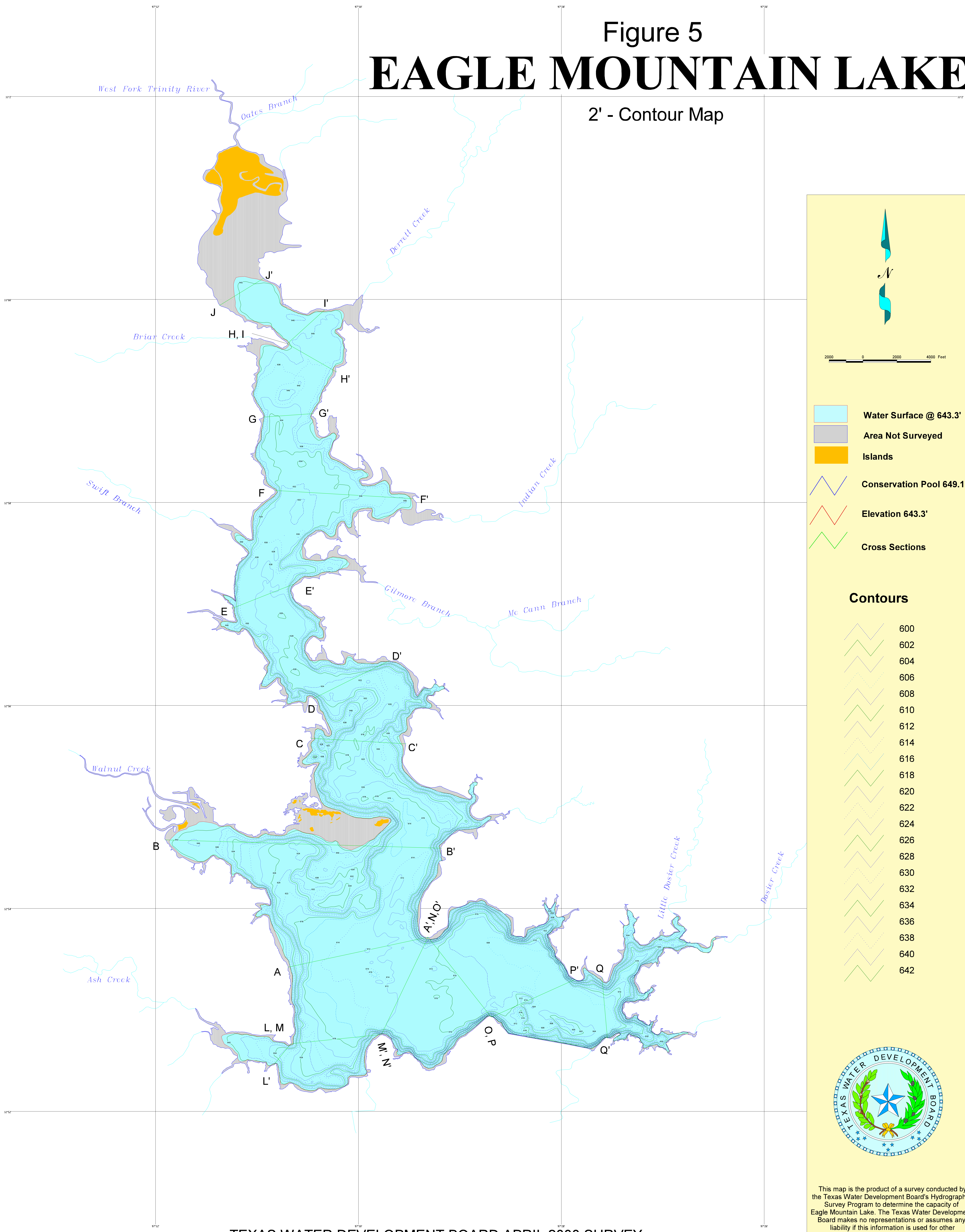
Depth Ranges from CPE

- 0' - 10'
- 10' - 20'
- 20' - 30'
- 30' - 40'
- 40' - 50.9'
- Islands @ 649.1
- Elevation 649.1
- Cross Sections

5000 0 5000 Feet

Figure 5 EAGLE MOUNTAIN LAKE

2' - Contour Map



- Water Surface @ 643.3'**
- Area Not Surveyed**
- Islands**
- Conservation Pool 649.1'**
- Elevation 643.3'**
- Cross Sections**

Contours

- 600
- 602
- 604
- 606
- 608
- 610
- 612
- 614
- 616
- 618
- 620
- 622
- 624
- 626
- 628
- 630
- 632
- 634
- 636
- 638
- 640
- 642

This map is the product of a survey conducted by the Texas Water Development Board's Hydrographic Survey Program to determine the capacity of Eagle Mountain Lake. The Texas Water Development Board makes no representations or assumes any liability if this information is used for other purposes such as boating maps.